



New Features of HEC-RAS 4.0

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New Features in HEC-RAS 4.0

- Overflow Gates
- User Defined Rules for Gate Operations
- Pressure Flow in Pipes
- Pump Station Rules
- Hager's Lateral Weir Equation
- Geo-referencing Tools
- Water Quality Temperature Modeling
- Sediment Transport (erosion and deposition)



Overflow Gates

• Open Air

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∼ c	ross Section - Warning Geometry is newer than output.	Gate Group: Gate #1 🔽 🖡 🖿 🛅 🏠 🗙 🖻
File	Options Help	Gate type (or methodology): Overflow (open air) 💌
River:	Reach 1 💽 🛌 🥘	Geometric Properties Weir Flow Over Gate
Reac	n: A 💌 River Sta.: 250 IS 💌 🖡 🕇	Height: 6 Weir Shape: Sharp Crested 💌
		Width: 6 Weir Method: User entered coefficient
	.04	Invert: 89 Weir Coefficient: 3.2
	95	# Openings: 2
		Centerline Stations
		Station
		_ <u>1 115.</u> 2 130.
£	90	
() ()		
Elevation (ft)		
	85	
		8
		10
	80	
	80 100 120 140 160	OK Cancel Help
4	Station (ft)	
4		

Inline Gate Editor

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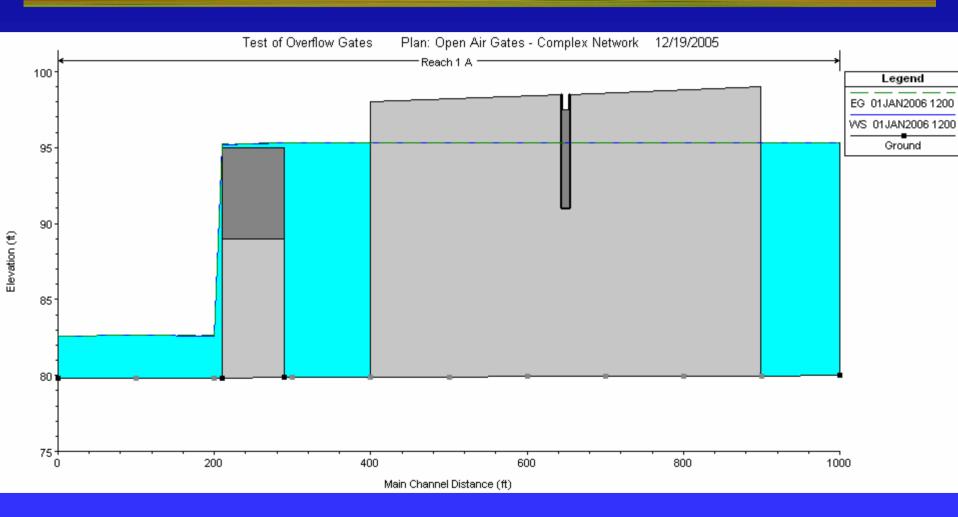
Overflow Gates

Closed Top		Inline Gate Editor			
	-	Gate Group: 🛛 Gate #1 💽 🗸 🖿 🖿 🚺 🖍 🖿			
	iss Section ptions Help	Gate type (or methodology): Overflow (closed top)			
	Harney T Harney	Geometric Properties Gate Flow Height: 4 Width: 4.7 Invert: 18.5			
Elevation (ft)		# Openings: 1 Station Submerged Orifice Flow 1 35. 2 Orifice Coefficient (typically 0.8): 3 Head Reference: 4 Sill (Invert) 4 Veir Flow Over Gate 6 Weir Shape: 7 Weir Method: 9 User entered coefficient 10 11 12 Veir Coefficient: 0K Cancel			
•	Station (ft)				





Overflow Gates Example



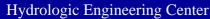




Operation Rules for Gated Structures

- Unsteady Flow Editor "Rules" boundary condition
- Inline/Lateral Structures
- Storage Area Connections
- Controls
 - Gates
 - Weir Coefficients
 - Min/Max Flow
- Rules are evaluated at every time step

Unsteady Flow Data - File Options Help	Unsteady	Flow with	ND target 22.65 wr	0 🔀
Boundary Conditions] Initial C	Conditions]			Apply Data
Select Location for Boundary Condition				
River: CowHouseCk	•			
Reach: 1		erSta.: 12	2500 👻 Add a Br	oundary Condition Location
neach: ji		J		bundary condition Eccation
Stage Hydrograph	Flow Hydro		dition Types Stage/Flow Hydr.	Rating Curve
Normal Depth	Lateral Inflo	w Hydr.	Uniform Lateral Inflow	Groundwater Interflow
T.S. Gate Openings	Elev Controlle	ed Gates	Navigation Dams	IB Stage/Flow
Rules				
River Reach	n	RS	Boundary Condition Type	
1 CowHouseCk	1	12500	Flow Hydrograph	
2 Harney	1	73.3 IS	Rules	
3 Hillsborough	1	605400	Flow Hydrograph	
4 Hillsborough 5 Hillsborough	1	605101.* 605100	Lateral Inflow Hydr. Lateral Inflow Hydr.	
6 Hillsborough	1	604999 IS		
7 Hillsborough	1	602447.*	Lateral Inflow Hydr.	
8 Hillsborough	1	602400	Lateral Inflow Hydr.	
9 Hillsborough	1b	601300	Lateral Inflow Hydr.	
10 Hillshorough	1h	601050	Lateral Inflow Hudr	<u> </u>
Storage Area and SA Conne	otions:		✓ Add a B	oundary Condition Location
Storage Area or SA Conne	ection		Boundary Condition Type	
1				
1.501.00.00.00.00.00.00.00.00.00.00.00.00.0				
Initial internal water surface elev	/ations set			







User Defined Rules Editor for Operating Gated Structures

Operation Rules

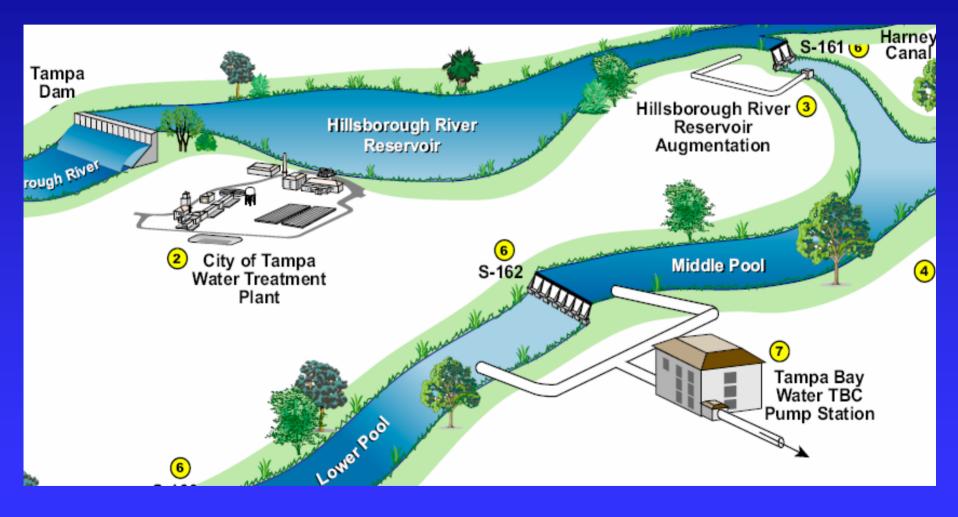
Rule Based Operations					
row Operation	True	False	^		
1 Real 'Tampa Dam Vol since midnight' (Initial Value = 0)	2	2			
2 Real 'S-161 Vol since midnight' (Initial Value = 0)	3	3			
3 Real 'S-161 Vol Diversion'	4	4			
4 'Tampa Dam 4 Hour Ave Flow' = Inline Structures: Structure - Total Flow (Fixed)(Hillsborough,2,600042,Average over previous time window,4,0)	5	5	-		
5 'Time Step hours' = Solution:Time Step(Value at current time step) 6 'Time Step seconds' = 3600 * 'Time Step hours'	5	ь 7	ľ		
7 'Tampa Dam Flow' = Inline Structures:Structure - Total Flow (Fixed)(Hillsborough,2,600042,Value at current time step)	8	8			
8 'S-161 Flow' = Inline Structures: Structure - Total Flow (Fixed)(Hamey,1,73.3,Value at current time step)	9 9	9			
9 'Tampa Dam Vol since midnight' = 'Tampa Dam Flow' * 'Time Step seconds' + 'Tampa Dam Vol since midnight'	10	10			
10 'S-161 Vol since midnight' = 'S-161 Flow' * 'Time Step seconds' + 'S-161 Vol since midnight'	11	11			
11 'Day Beg time step' = Time:Day of Month(Begining of time step)	12	12			
12 'Day End time step' = Time:Day of Month(End of time step)	13	13			
13 If ('Day Beg time step' <> 'Day End time step') Then 14 'HR 24hour ave Flow' = 'Tampa Dam Vol since midnight' + 'S-161 Vol since midnight' / 86400	14 15	50 15			
14 Hh 24hour ave Flow = Tampa Dam vol since mignight + 3-161 vol since mignight / 86400 15 'Tampa Dam Vol since midnight' = 0	16	16			
15 Tampa Dam Vol since midnight = 0 16 S-161 Vol since midnight = 0		17	~		
Insert New Operation					
Comment New Variable Get Sim Value Set Operational Param Branch (If/Else) Math Table Copy I I Disable					
- Get Simulation Value					
Assign Result Alue at current time step	1	-			
C Existing Variable Structure - Total Flow (Fixed)	-	-			
Churchurg Tabal Flow (Desired)					
(• New Variable Structure - Flow Additional					
S-161 Flow Structure - Flow Maximum RS: 73.3 IS					
Structure - Flow Minimum					
Structure - Total Gate Flow					
Structure - Total Gate Flow Maxi					
Structure - Total Gate Flow Minin					
Structure Cate Master Catting					
(Simulation variables in bold are only available for the current structure)					
OK	ſ	Connel			

OK Cancel





Tampa Bay Water System Overview







TBW S-161 Diversion Structure Rules

- Get previous 24 hour outflow
 - Outflow includes Tampa Dam & S-161
- Determine allowable diversion:

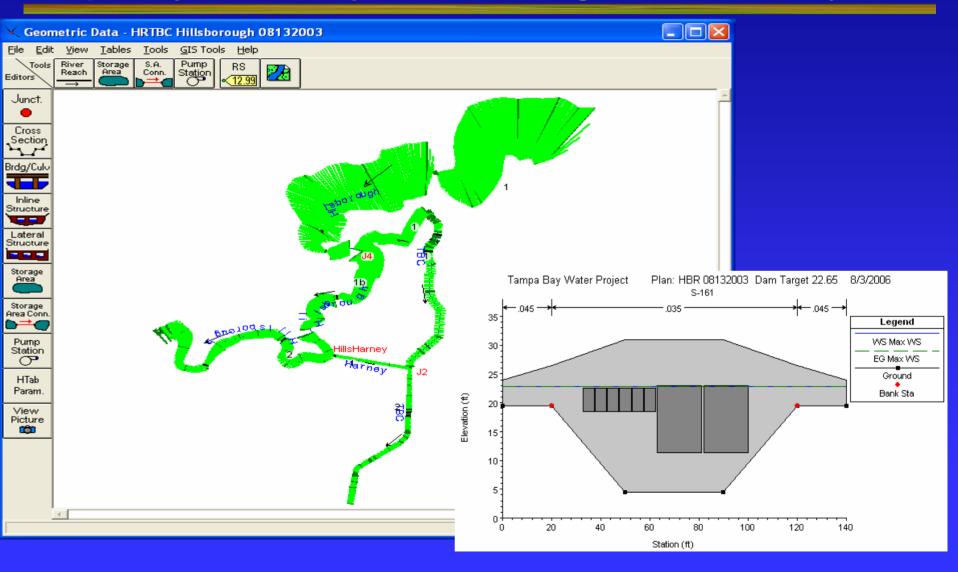
Discharge at Tampa Dam (mgd)	Withdrawal from Middle Pool (mgd)
Less than 65	0
65-97	10% of the discharge at Tampa Dam
97-139	10-30% of the discharge
139-647	30% of the discharge
More than 647	194

- Adjust S-161 gates to get allowable diversion in ~20 hours
- Close gates when/if:
 - Maximum volume diverted
 - 4 hour running average at Tampa Dam < 10cfs



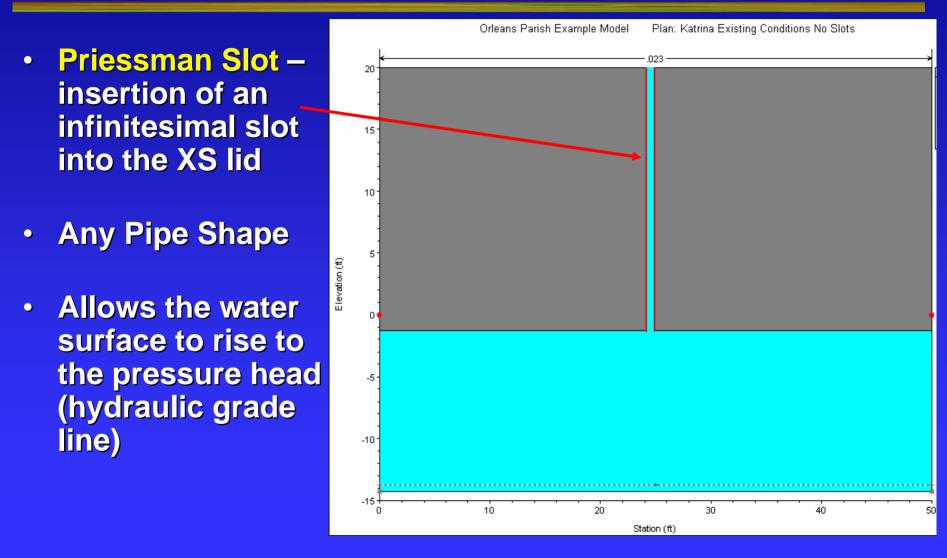


Animation of Gate Operations Tampa Bay Water Project Hillsborough River – Harney Canal





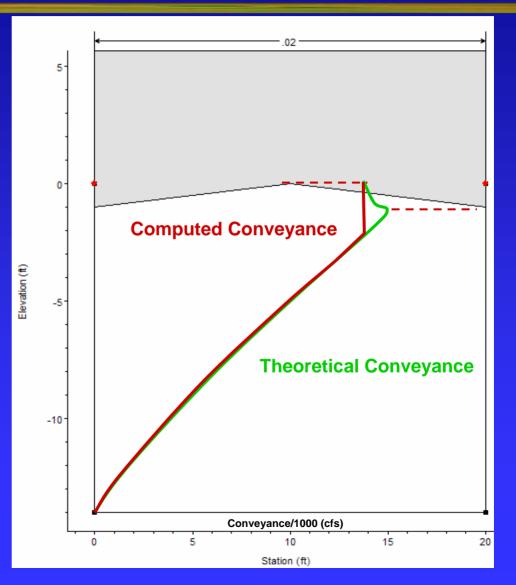
Pressurized Pipe Flow





Pressurized Pipe Flow

- Conveyance and wetted perimeter are cut off at top of pipe
- Area is added, but it is negligible
- Conveyance curve is truncated to local minimum to increase stability



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Pump Station Override Rules

Pump Station Data Editor				
Pump Station Name: Pump15 💽 🚽 🕇 Rename Pump Station				
Pump Connection Data Pump Group Data Advanced Control Rules				
Add New Rule Delete Rule Copy Rule 🖡 🕇				
Pump Rules				
Day/Hour based rule - flow max = 0 start at: 28AUG 0000 end at: 28AUG 1330 Day/Hour based rule - flow max = 250 start at: 28AUG 1330 end at: 28AUG 1530				
Day/Hour based rule - flow max = 750 start at: 28AUG 1530 end at: 28AUG 1545 Day/Hour based rule - flow max = 500 start at: 28aug 1545 end at: 2aug 1600				
Day/Hour based rule - flow max = 0 flow min = 0 start at: 28AUG 1600 end at: 13SEP 0900				
Edit Current Selected Rule Bule Flow Maximum: 250 Bule Flow Minimum:				
Transition (min)				
Transition (min): 5				
Rule Start Day: 28AUG Rule Start Hour: 1330				
Rule End Day: 28AUG Rule End Hour: 1530				
Plot Pump Curves OK Cancel				



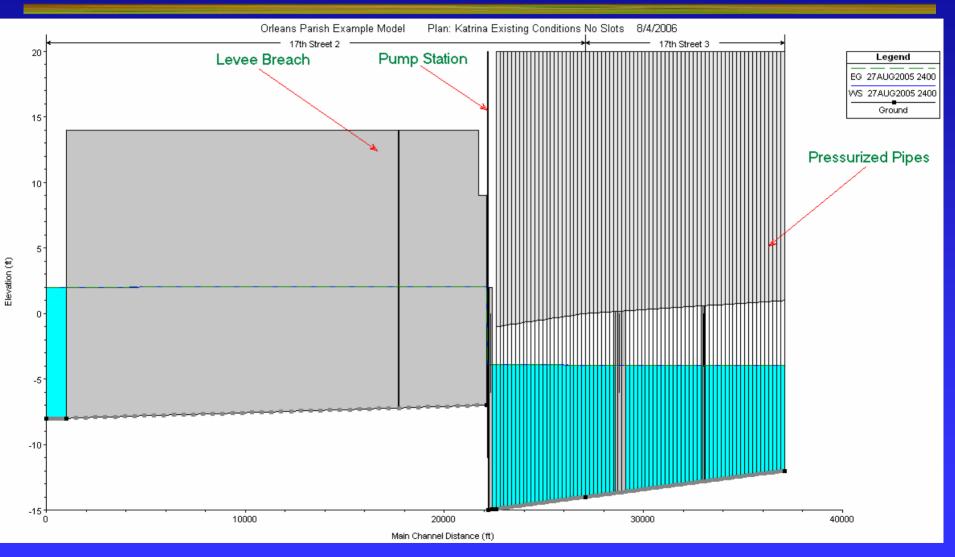
New Orleans - 17th Street Pump Station

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Pressurized Pipes, Pump Station, And Levee Breach Animation







Hager's Lateral Weir Equation

$Q = CLH^{3/2}$

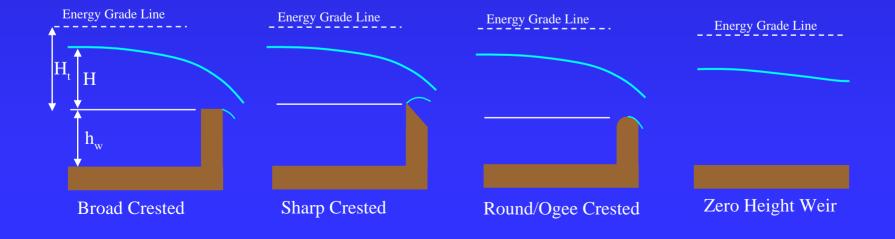
$$C = \frac{3}{5}C_0\sqrt{g} \left[\frac{1-W}{3-2y-W}\right]^{0.5} \left\{1-(\beta+S_0)\left[\frac{3(1-y)}{y-W}\right]^{0.5}\right\}$$

w

w

$$W = \frac{h_w}{H_t + h_w} \qquad y = \frac{H + h_w}{H_t + h_w}$$

$$C_0 = Function(weir shape)$$

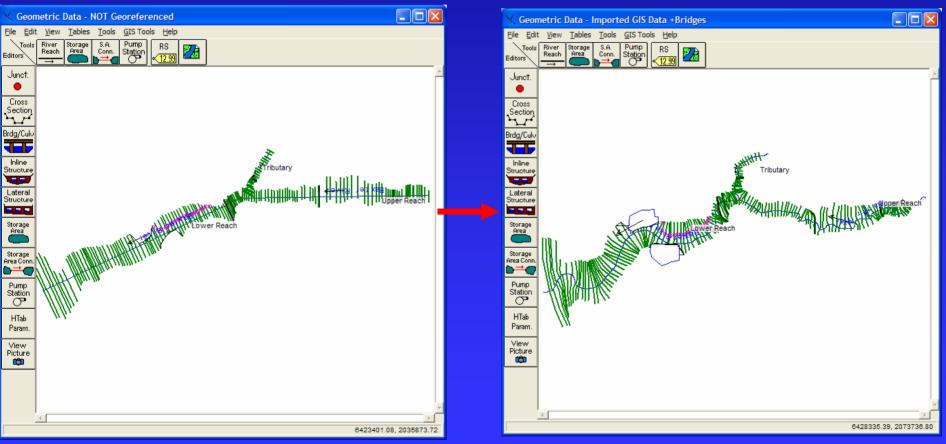






Geo-referencing Tools in HEC-RAS

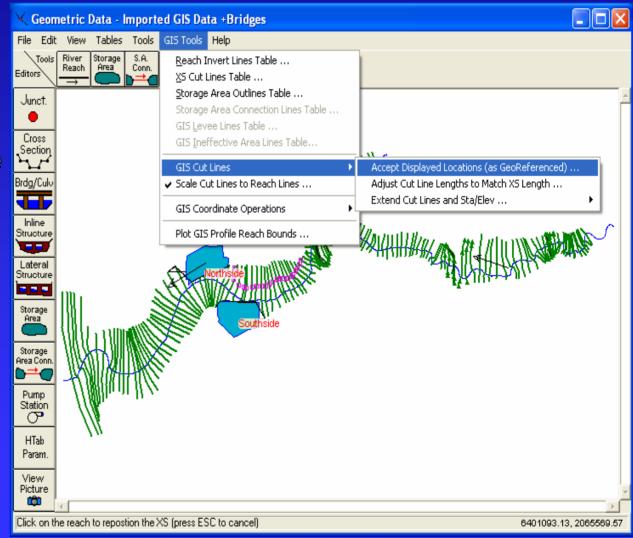
From "stick figure" to real locations





Geo- referencing Tools in HEC-RAS

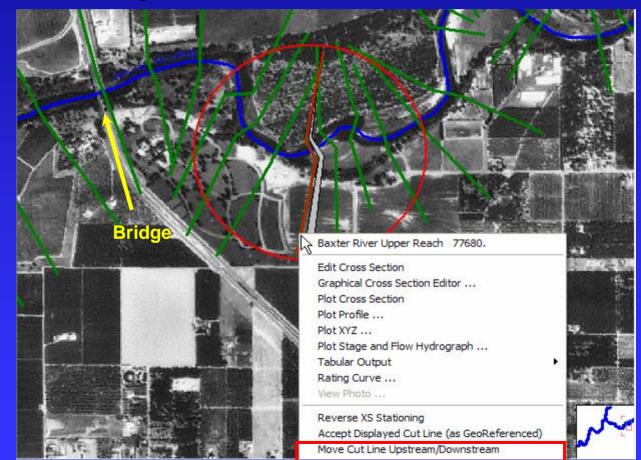
- Fix the cross sections at "known" locations
- RAS will help move the rest of the sections





Geo-referencing

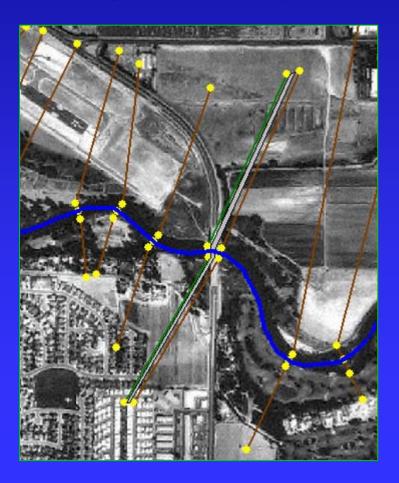
Move Cut Line Upstream/Downstream

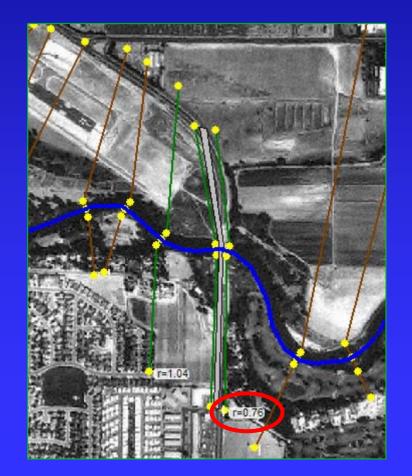




Geo-referencing

Edit | Move Object









Geo-referencing: New XS Interpolation

Bank to bank perpendicular

Georeferenced XS

Left and right overbanks perpendicular to respective widths







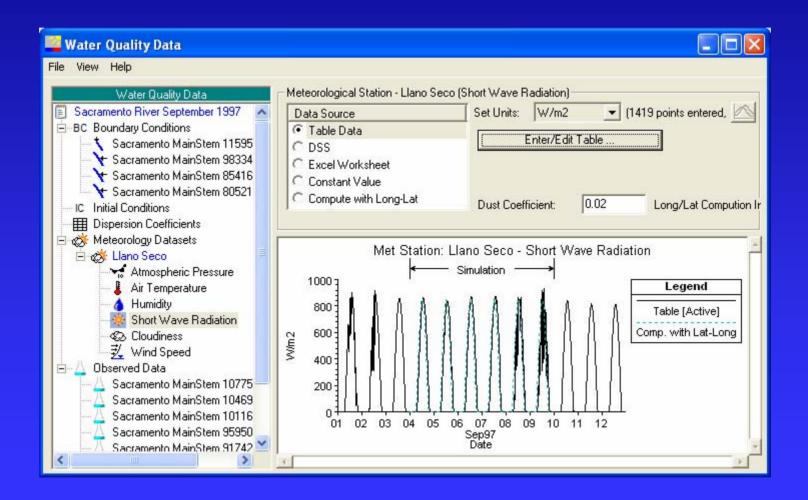
Water Quality (Temperature) Model

- Based on unreleased version of CE-QUAL-RIV1
- Numerical Scheme
 - Finite Volume
 - Variable grid size
 - Automatic time step selection
- Full energy budget





Meteorological Data Editor – Solar Radiation







Source/Sink Term for Temperature (Energy Budget)

solar radiation (qsw)

f (site location, time of day, day of year, atmospheric turbidity, cloud cover)

net longwave radiation (qlw) f (air temperature, water temperature)

sensible heat (qh) f (temperature gradient, wind, a&b)

latent heat (qe) f (vapor pressure gradient, wind, a&b)

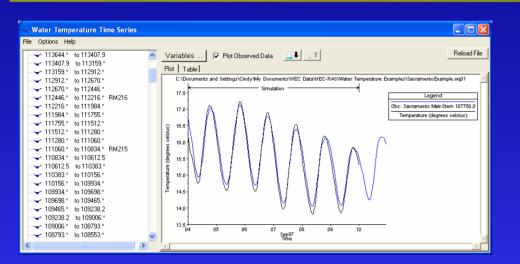
$$q_{net} = q_{sw} + q_{lwn} + q_h + q_e$$

Planned:

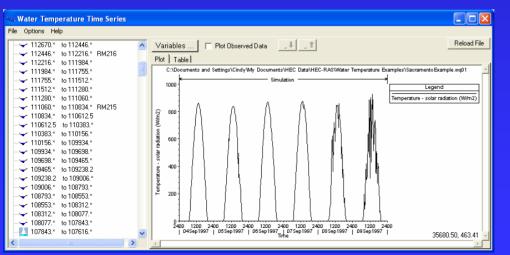
- ground heat conduction
- shading (topographic, riparian)



Time Series Plots



Water temperature

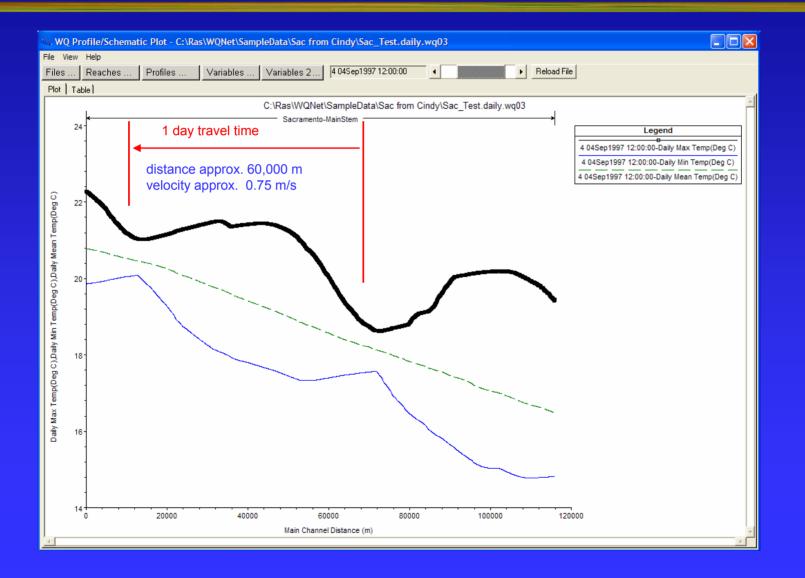


Solar Radiation





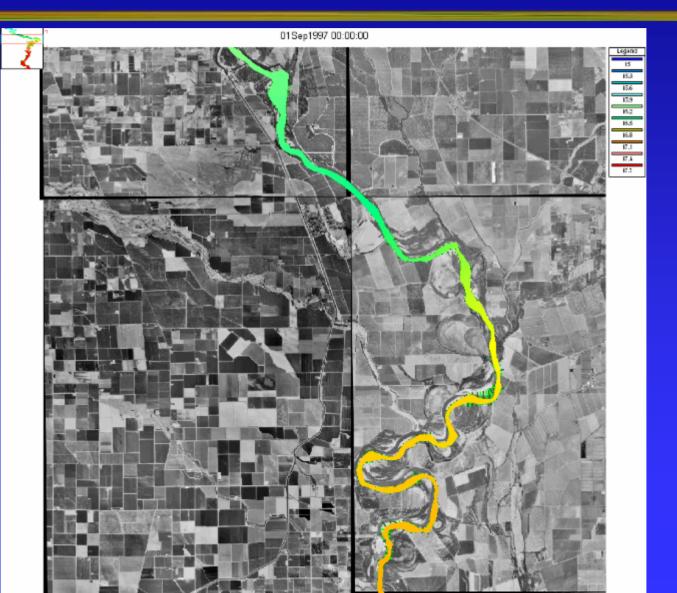
Profile Plot of Temperature







Map View







Mobile Bed Sediment Transport

- Quasi-Steady Hydrodynamics
- Transport Capacity
- Sediment continuity
- Sorting and Armoring
- Erosion and Deposition
- Graphical User Design



Transport Potential Functions

- Ackers-White
- •Englund-Hansen
- Laursen (Copland)
- •Myer-Peter-Meuler
- Toffaleti
- •Yang (Sand and Gravel)
- Wilcock





Transport Capacity by Multiple Grain Sizes

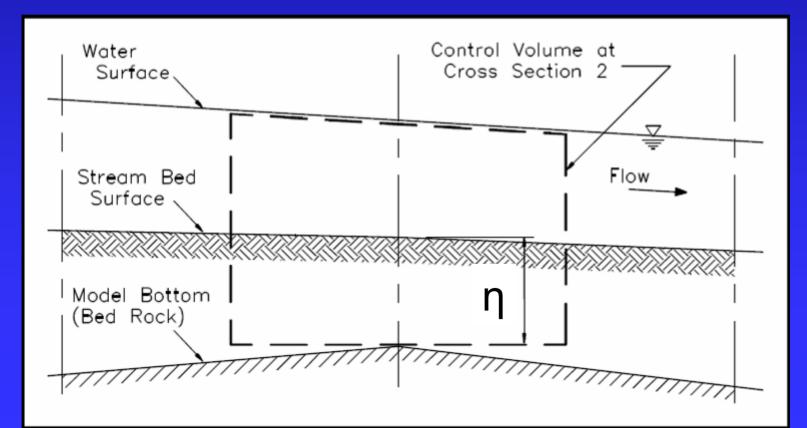
- Bed Material and Inflowing Load divided into separate grain classes (up to 20)
- Transport potential is calculated for each grain size
- Transport Capacity = (Transport Potential for each grain size) X (fraction of that material in active layer of bed)





Sediment Continuity: Exner Equation











Temporal Constraints on Eroding and Constraints on Eroding

- Erosion and deposition does not occur instantaneously.
- Deposition is based on settling velocity:
 - Deposition efficiency coefficient =

 $\frac{V_s(i) \cdot \Delta t}{V_s(i)}$

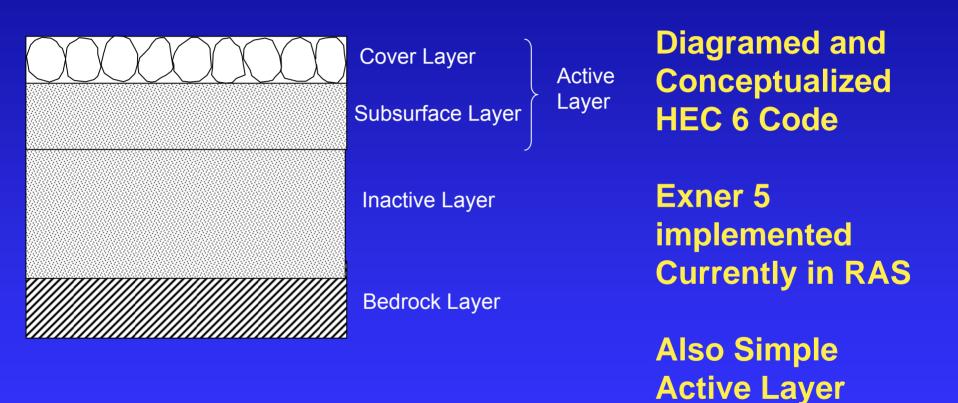
- Erosion is based on "Characteristic Flow Length"
 - Erosion = (Gs Qs) x Ce Entrainment Coefficient
 - Where:

$$C_{e} = 1.368 - e^{\frac{L}{30 \cdot D}}$$

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Sorting and Armoring



Erosion can be further constrained by the cover Layer

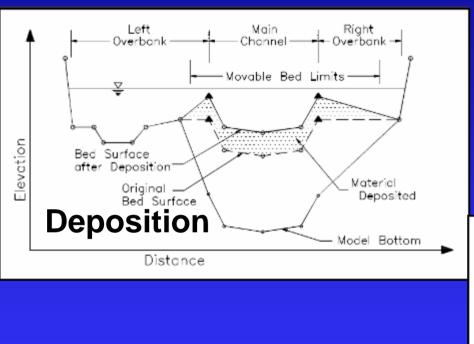
Method

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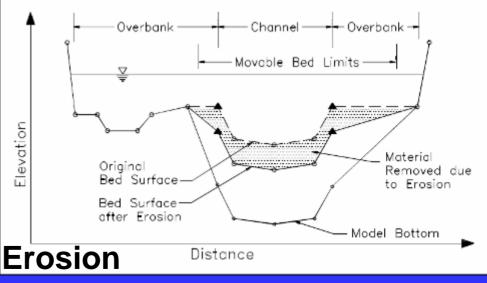




Erosion and Deposition to RAS Cross Sections



RAS computations modified to compute bed changes and modify cross sections before each time step

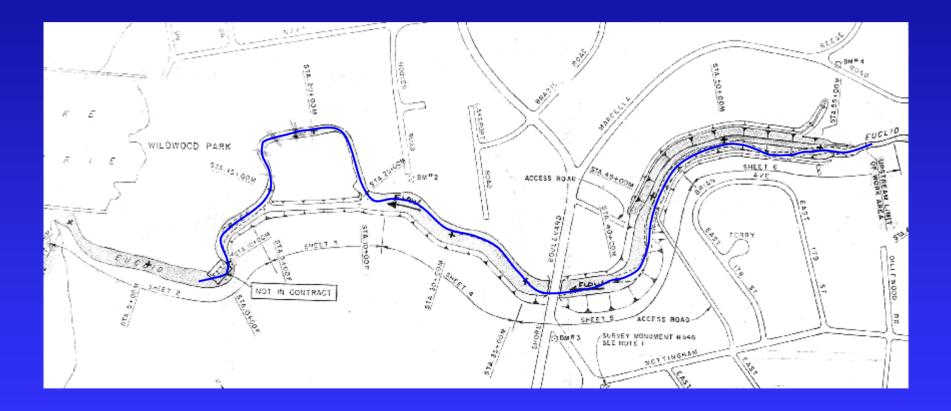


•Cross Sections •Bridges





Example Application: Euclid Creek





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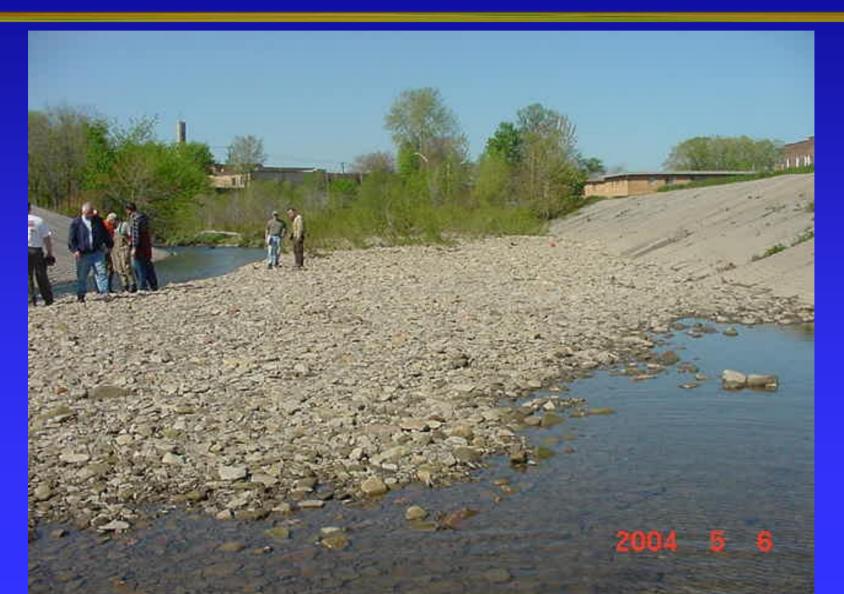
Case Study: Euclid Creek



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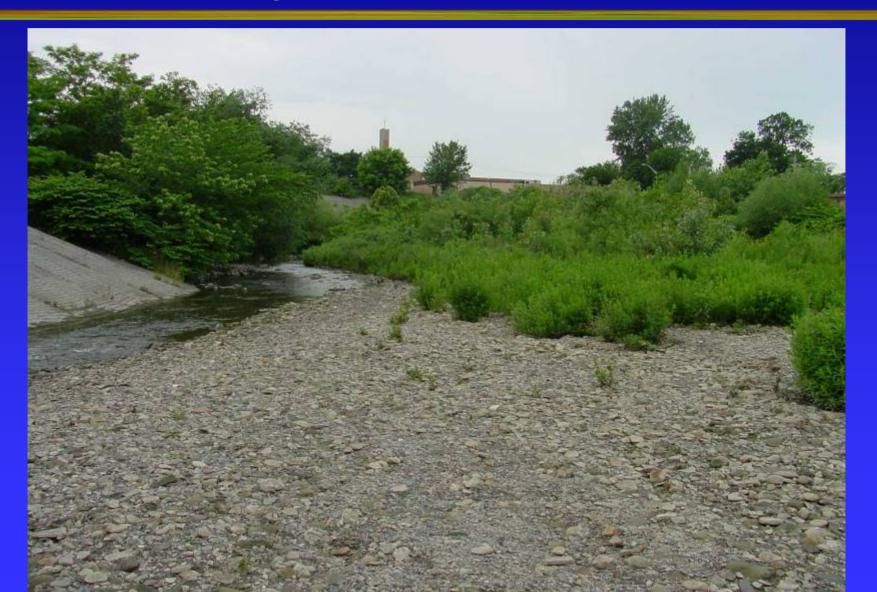
Case Study: Euclid Creek



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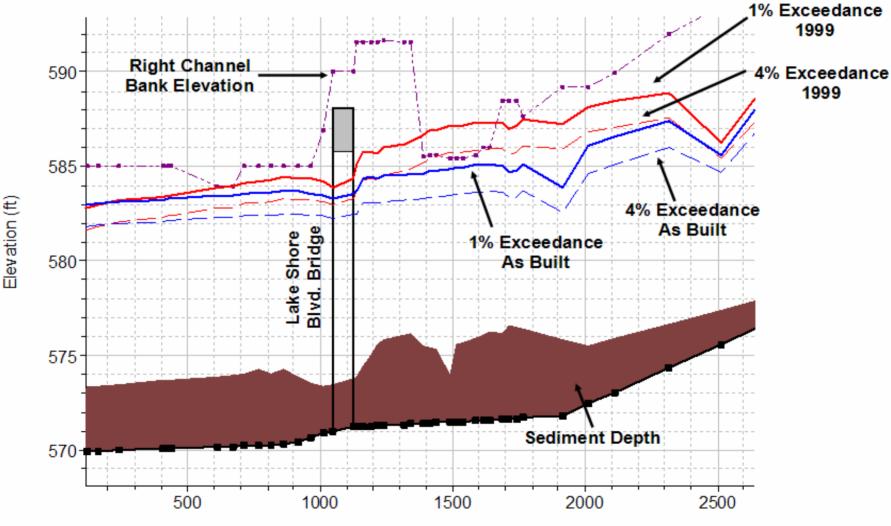
Case Study: Euclid Creek



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Case Study: Euclid Creek



Main Channel Distance (ft)





Animation of Bed Movement

