

Allowed SF = P_{cr}

Chapter 4 Equations

Axial Tensile Strain

$$\epsilon = \frac{\Delta L}{L}$$

Torsional shear modulus

$$G = \frac{E}{2(1+\nu)}$$

Factor by which compressive strength is reduced because of buckling tendencies

Euler range

$$\alpha = \frac{S_y}{S_{cr}} = \frac{S_y (Lc/p)^2}{\pi^2 E}$$

Johnson range

$$\alpha = \frac{S_y}{S_{cr}} = \frac{4\pi^2 E}{4\pi^2 E - S_y (Lc/p)^2}$$

Column buckling; elastic instability

Euler

$$P_{cr} = \frac{\pi^2 EI}{Lc^2}$$

$$S_{cr} = \frac{P_{cr}}{A} = \frac{\pi^2 E}{(Lc/p)^2} \text{ or } \frac{S_{cr}}{E} = \frac{\pi^2}{(Lc/p)^2}$$

rectangle p = 20th, round solid, p = D/4
L = slenderness ratio, radius of gyration

Johnson

$$S_{cr} = \frac{P_{cr}}{A} = S_y - \frac{S_y^2}{4\pi^2 E} \left(\frac{Lc}{p}\right)^2$$

Stress concentrations

$$\sigma_{max} = K_t \sigma_{nom}$$

Axial tensile stress

$$\sigma = P/A \tag{4.1}$$

Average direct shear stress

$$\tau = P/A \tag{4.2}$$

Torsional shear stress at radius r (round bar)

$$\tau = Tr/J \tag{4.3}$$

Surface torsional shear stress for round bar of diameter d

$$\tau = 16T/\pi d^3 \tag{4.4}$$

Maximum torsional shear stress for rectangular bar

$$\tau_{max} = T(3a + 1.8b)/a^2 b^2 \tag{4.5}$$

Normal stresses for beam loaded in bending

$$\sigma = My/I \tag{4.6}$$

Maximum normal stresses for beam loaded in bending (section modulus Z)

$$\sigma_{max} = M/Z \tag{4.7}$$

Maximum normal stresses for solid round bar loaded in bending

$$\sigma_{max} = 32M/\pi d^3 \tag{4.8}$$

Tensile and compressive stresses for curved beam in bending

$$\sigma_i = + \frac{Mc_i}{eAr_i} \text{ and } \sigma_o = - \frac{Mc_o}{eAr_o} \tag{4.9}$$

Distance between the neutral axis and the centroidal axis

$$e = \bar{r} - \frac{A}{\int dA/p} \tag{4.10}$$

Tensile and compressive stresses for curved beam in bending

$$\sigma_i = K_t Mc/I = K_t M/Z \text{ and } \sigma_o = -K_o Mc/I = -K_o M/Z \tag{4.11}$$

Transverse shear stress in beam

$$\tau = \frac{V}{Ib} \int_{y=y_0}^{y=c} y dA \quad \frac{VQ}{I\pm} \tag{4.12}$$

Maximum transverse shear stress in beam (solid round section)

$$\tau_{max} = \frac{4}{3} V/A \tag{4.13}$$

Maximum transverse shear stress in beam (solid rectangular section)

$$\tau_{max} = \frac{3}{2} V/A \tag{4.14}$$

$F_t = ma$
 $W = \int_0^s F ds = F \cdot s = T \theta = \frac{\text{Total (kW)}}{1000 \cdot 60}$
spinning
 $\dot{W} = F \cdot V \quad 550 \frac{\text{ft} \cdot \text{lb}}{\text{HP} \cdot \text{s}}$
 $\dot{W} = T \omega$
 $\omega = \frac{V}{R} \quad A = \frac{PL}{AE} \quad \theta = \frac{T \rho}{JG}$
 $1 \text{ lbm} = \frac{1}{32.2} \text{ slug}$

$$\tau = \frac{VQ}{It}$$

(Chapter 4 Equations Continued)

Maximum transverse shear stress in beam (thin-wall tubing)

$$\tau_{max} = 2V/A \quad (4.15)$$

Principal normal stresses

$$\sigma_1, \sigma_2 = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\tau_{xy}^2 + \left(\frac{\sigma_x - \sigma_y}{2}\right)^2} \quad (4.16)$$

Principal directions—angle between the principal axes and the x and y axes (or the angle between the principal planes and the x and y planes)

$$2\phi = \tan^{-1} \frac{2\tau_{xy}}{\sigma_x - \sigma_y} \quad (4.17)$$

Maximum shear stress

$$\tau_{max} = \pm \sqrt{\tau_{xy}^2 + \left(\frac{\sigma_x - \sigma_y}{2}\right)^2} \quad (4.18)$$

Normal stress on a plane oriented at angle ϕ from the #1 principal plane

$$\sigma_\phi = \frac{\sigma_1 + \sigma_2}{2} + \frac{\sigma_1 - \sigma_2}{2} \cos 2\phi \quad (4.19)$$

Shear stress on a plane oriented at angle ϕ from the #1 principal plane

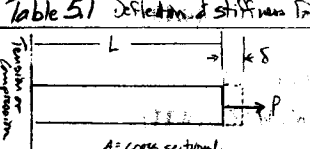
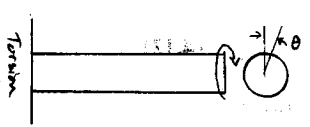



$$\tau_\phi = \frac{\sigma_1 - \sigma_2}{2} \sin 2\phi \quad (4.20)$$

Maximum normal stress; maximum shear stress

$$\sigma_{max} = K_t \sigma_{nom} \quad \text{and} \quad \tau_{max} = K_t \tau_{nom} \quad (4.21)$$

Strain for unrestrained homogeneous, isotropic body

$$\epsilon = \alpha \Delta T \quad (4.22)$$

Table 5.1 Deflection and stiffness for straight bars (beams) of uniform area		Static
	Deflection	$\sum F = 0 \quad \sum M = 0$
Tension or Compression 	$\delta = \frac{PL}{AE}$ $k = \frac{P}{\delta} = \frac{AE}{L}$	Dynamics $\sum F = ma \quad \sum M = I\alpha$
Torsion 	$\theta = \frac{TL}{KJ}$ $K = \frac{T}{\theta} = \frac{KJ}{L}$ <small>(K = J = I_p d^4 / 32 for solid round bar)</small>	reactions & expressions for moments
Bending (Angular) 	$\theta = \frac{ML}{EI}$ $K = \frac{M}{\theta} = \frac{EI}{L}$	
Bending (Linear) 	$\delta = \frac{ML^2}{2EI}$ $k = \frac{M}{\delta} = \frac{2EI}{L^2}$	
Compressive Beams (Load @ End) 	$\delta = \frac{PL^3}{3EI}$ $k = \frac{P}{\delta} = \frac{3EI}{L^3}$	

Castigliano's Method

- 1) identify sources of elastic nrg (axial, bending, torsion, transverse)
- 2) apply dummy load in direction of desired deflection if necessary
- 3) express, in general terms, internal loads over its whole length
- 4) Find all the partials (necessary) of internal loads
- 5) set dummy loads = to 0
- 6) Integrate