

Sediment Mechanics

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River Mechanics and Sediment Transport
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Objectives

Brief overview of examples and techniques:

1. Fall Velocity;
2. Turbulent Velocity Profiles;
3. Incipient Motion;
4. Bedforms.

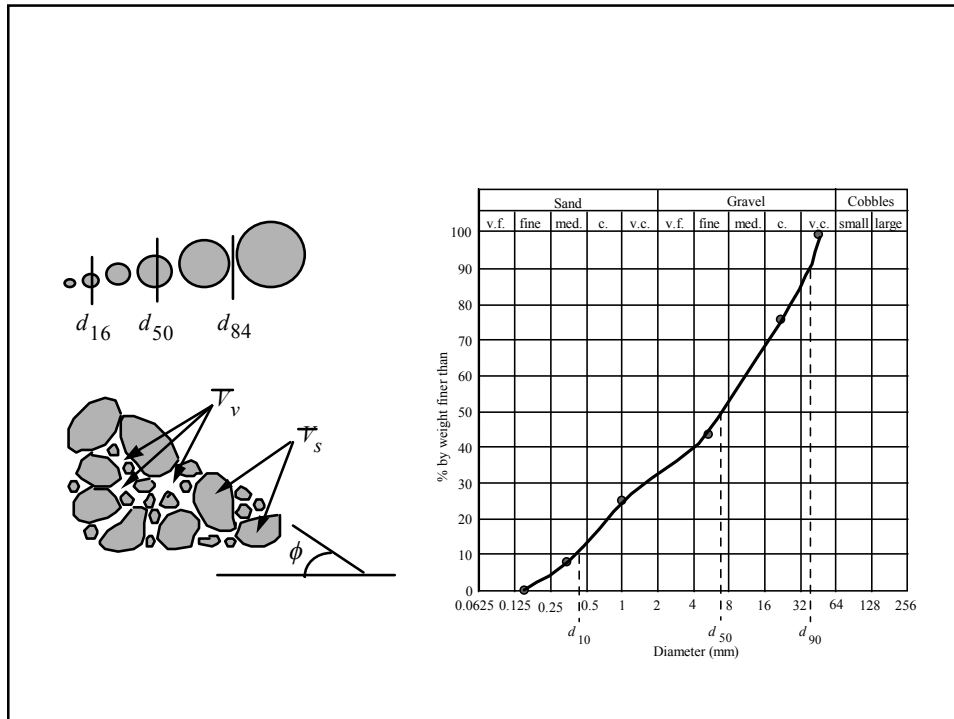
1. Fall Velocity

			Openings Per Inch		Class	
Millimeters	Microns	Inches	Tyler	U.S. Standard		
4000-2000	—	—	160-80	—	Very large boulders	
2000-1000	—	—	80-40	—	Large boulders	
1000-500	—	—	40-20	—	Medium boulders	
500-250	—	—	20-10	—	Small boulders	
250-130	—	—	10-5	—	Large cobbles	
130-64	—	—	5-2.5	—	Small cobbles	
64-32	—	—	2.5-1.3	—	Very coarse gravel	
32-16	—	—	1.3-0.6	—	Coarse gravel	
16-8	—	—	0.6-0.3	2 1/2	Medium gravel	
8-4	—	—	0.3-0.16	5	Fine gravel	
4-2	—	—	0.16-0.08	9	Very fine gravel	
2-1	2.00-1.00	2000-1000	—	16	18	Very coarse sand
1-1/2	1.00-0.50	1000-500	—	32	35	Coarse sand
1/2-1/4	0.50-0.25	500-250	—	60	60	Medium sand
1/4-1/8	0.25-0.125	250-125	—	115	120	Fine sand
1/8-1/16	0.125-0.062	125-62	—	250	230	Very fine sand
1/16-1/32	0.062-0.031	62-31	—			Coarse silt

64-32	—	—	2.5-1.3	—	—	Very coarse gravel
32-16	—	—	1.3-0.6	—	—	Coarse gravel
16-8	—	—	0.6-0.3	2 1/2	—	Medium gravel
8-4	—	—	0.3-0.16	5	5	Fine gravel
4-2	—	—	0.16-0.08	9	10	Very fine gravel
2-1	2.00-1.00	2000-1000	—	16	18	Very coarse sand
1-1/2	1.00-0.50	1000-500	—	32	35	Coarse sand
1/2-1/4	0.50-0.25	500-250	—	60	60	Medium sand
1/4-1/8	0.25-0.125	250-125	—	115	120	Fine sand
1/8-1/16	0.125-0.062	125-62	—	250	230	Very fine sand
1/16-1/32	0.062-0.031	62-31	—			Coarse silt
1/32-1/64	0.031-0.016	31-16	—			Medium silt
1/64-1/128	0.016-0.008	16-8	—			Fine silt
1/128-1/256	0.008-0.004	8-4	—			Very fine silt
1/256-1/512	0.004-0.0020	4-2	—			Coarse clay
1/512-1/1024	0.0020-0.0010	2-1	—			Medium clay
1/1024-1/2048	0.0010-0.0005	1-0.5	—			Fine clay
1/2048-1/4096	0.0005-0.0002	0.5-0.24	—			Very fine clay

Los Corales





Fall Velocity

h

$$\omega = \frac{g(G-1)d_s^2}{18\nu} \quad \text{Stokes Law} \rightarrow \text{valid for silts \& clays}$$

$$\omega = \sqrt{\frac{4}{3} \frac{g(G-1)d_s}{C_D}} \quad \text{valid for gravels, cobbles \& boulders}$$

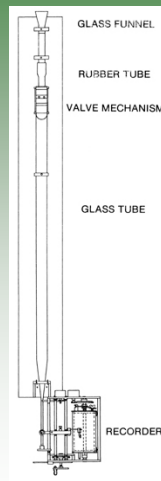
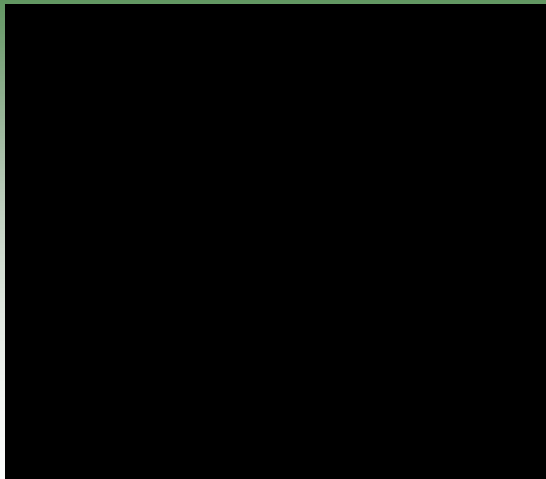
$$\omega = \frac{8v_m}{d_s} \left[\left(1 + \frac{d_s^3}{72} \right)^{0.5} - 1 \right]$$

dimensionless particle diameter

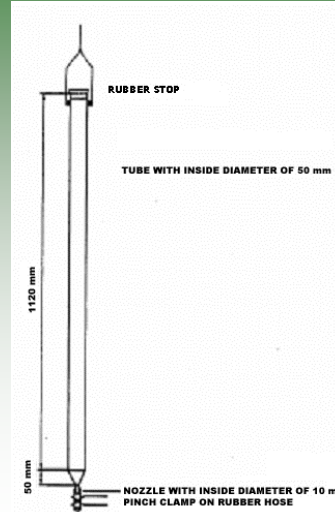
$$d_* = d_s \left[\frac{(G-1)g}{\nu_m^2} \right]^{\frac{1}{3}}$$



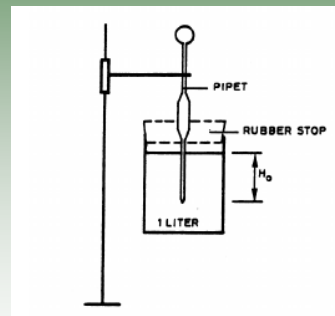
Visual Accumulation Tube method VAT



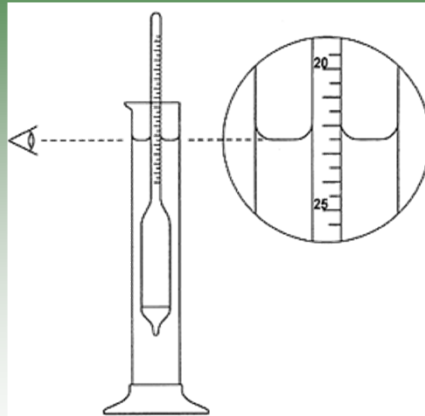
Bottom Withdrawal Tube method BWT



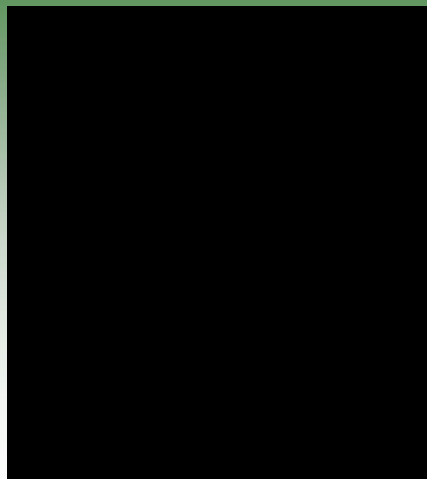
Pipette Method



Hydrometer Method



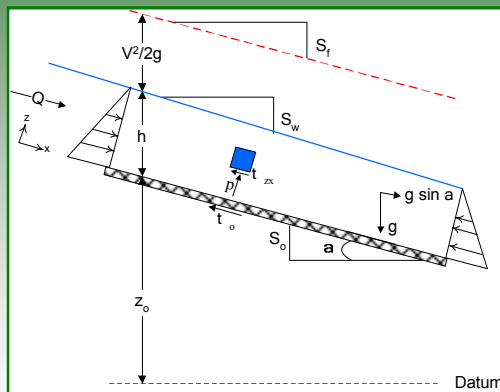
Example - Fall Velocity



$$d_s = d_s \left[\frac{(G-1)g}{v_m^2} \right]^{\frac{1}{3}}$$
$$d_s = 0.001182m \left[\frac{(2.65-1)9.81 \frac{m}{s^2}}{(1.14 \cdot 10^{-6} \frac{m^2}{s})^{\frac{1}{3}}} \right]^{\frac{1}{3}}$$
$$d_s = 27.4$$
$$\omega = \frac{8v_m}{d_s} \left[\left(1 + \frac{d_s^3}{72} \right)^{0.5} - 1 \right]$$
$$\omega = \frac{8 \cdot 1.14 \cdot 10^{-6} \frac{m^2}{s}}{0.001182m} \left[\left(1 + \frac{27.4^3}{72} \right)^{0.5} - 1 \right]$$
$$\omega = 0.123 \frac{m}{s}$$

2. Turbulent Velocity Profiles

Open Channel Flow



$$\int_p^0 dp = \rho \int_z^h -g dz$$

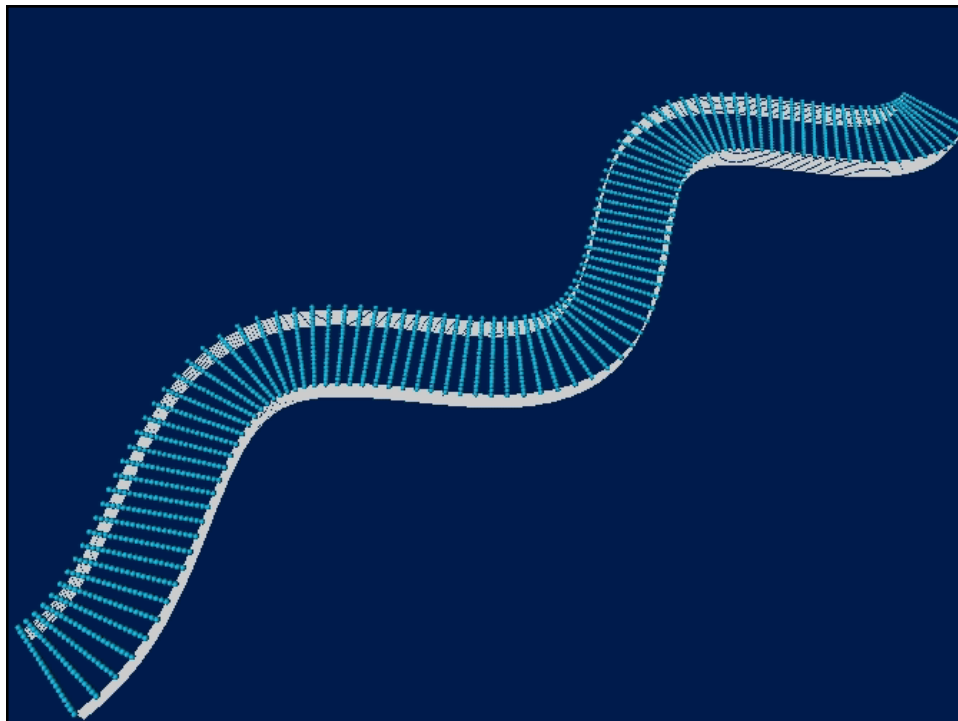
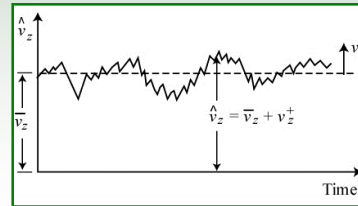
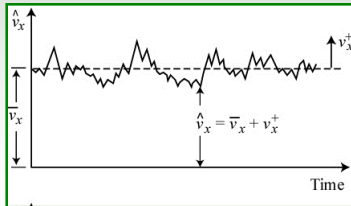
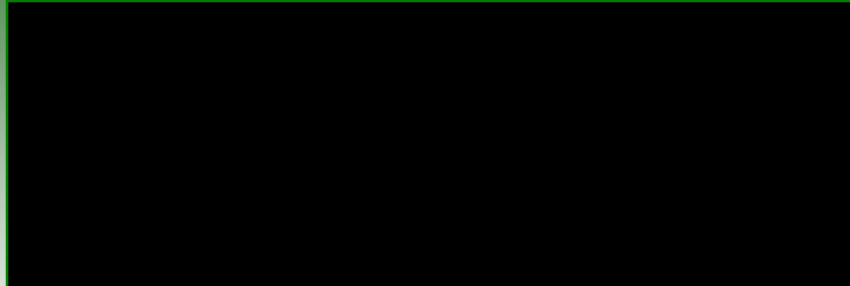
$$p = \rho g(h - z)$$

$$\tau_{zx} = \rho g(h - z) S_f$$

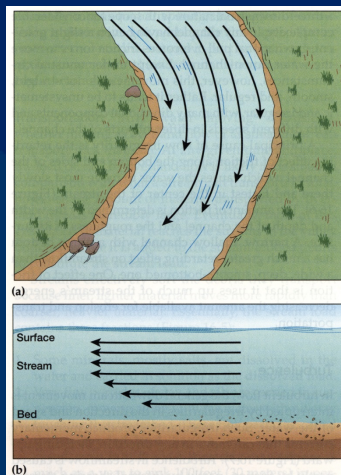
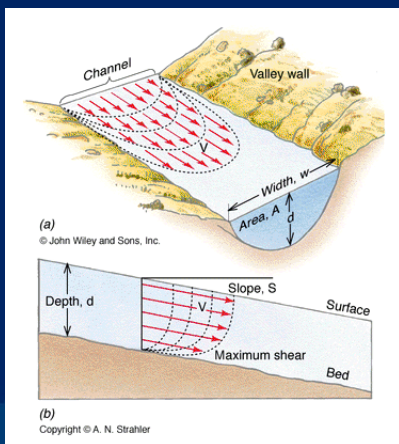
$$\tau_o = \rho g h S_f$$

$$u_* = \sqrt{\frac{\tau_o}{\rho}} = \sqrt{g h S_f}$$

Turbulent Flow Equations

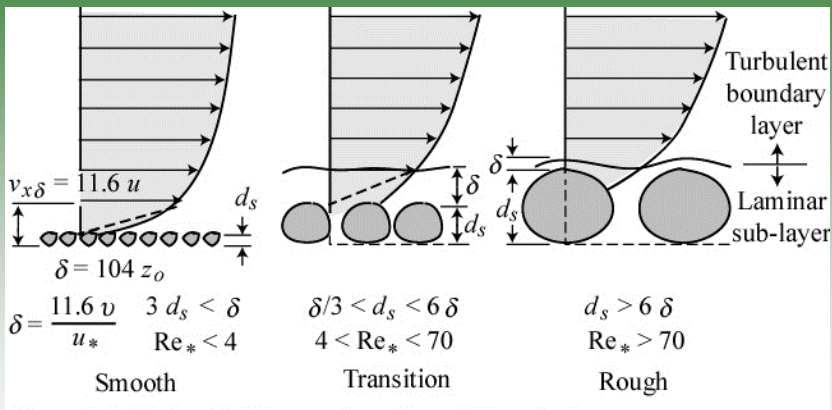


Logarithmic Velocity Profile



$$\frac{v_x}{u_*} = \frac{1}{\kappa} \ln \frac{z}{z_o}$$

Logarithmic Velocity Profile



$$v = \frac{u_*}{\kappa} \ln \left(9.05 \frac{z u_*}{\nu} \right)$$

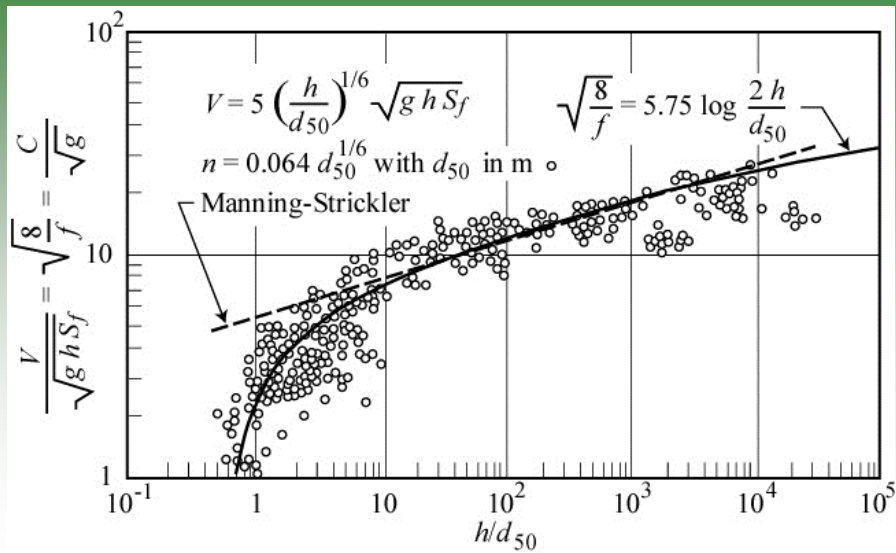
$$v = \frac{u_*}{\kappa} \ln \left(30.2 \frac{z \chi}{k_s} \right)$$

$$v = \frac{u_*}{\kappa} \ln \left(30.2 \frac{z}{k_s} \right)$$

Depth Average Velocity



Resistance to Flow



Saint-Venant Equation

Bed Slope

$$S_o = -\frac{\partial z_o}{\partial x}$$

Free Surface Slope

$$S_w = -\frac{\partial(z_o + h)}{\partial x} = S_o - \frac{\partial h}{\partial x}$$

Energy Slope

$$S_f = -\frac{\partial\left(z_o + h + \frac{V^2}{2g}\right)}{\partial x} = S_o - \frac{\partial h}{\partial x} - \frac{V\partial V}{g\partial x}$$

Unsteady Flow Saint-Venant Equation

$$S_f = S_o - \frac{\partial h}{\partial x} - \frac{V\partial V}{g\partial x} - \frac{\partial V}{g\partial t}$$

Example – Rhine River

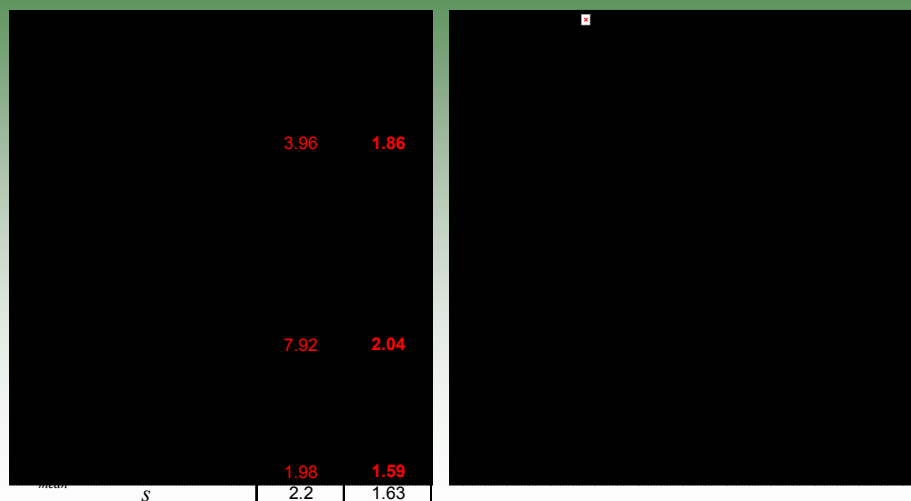
Depth	Velocity	Depth	Velocity
m	m/s	m	m/s
0.3	0.74	2.2	1.63
0.3	0.81	3.5	1.92
0.3	0.72	4	1.85
0.4	0.47	4.1	1.86
0.5	0.84	5.3	1.99
0.8	1.22	6	1.98
0.9	1.34	7.3	2.08
1.2	1.38	8	2.04
1.3	1.47	9	1.9

Example - Shear Velocity

$$u_* = \sqrt{ghS_f} = \sqrt{9.81 \frac{m}{s^2} * 9.9m * 0.0001312}$$

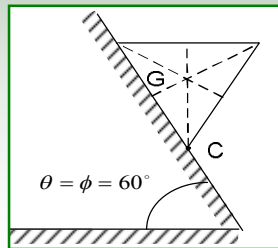
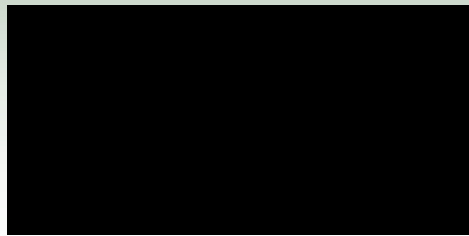
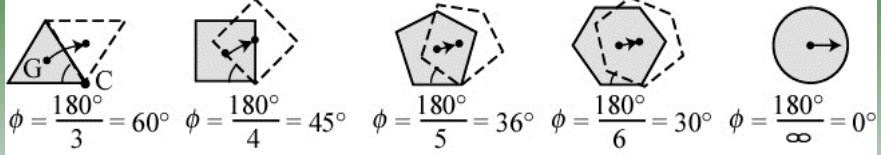
$$u_* = 0.113 \frac{m}{s}$$

Example - Mean Velocity

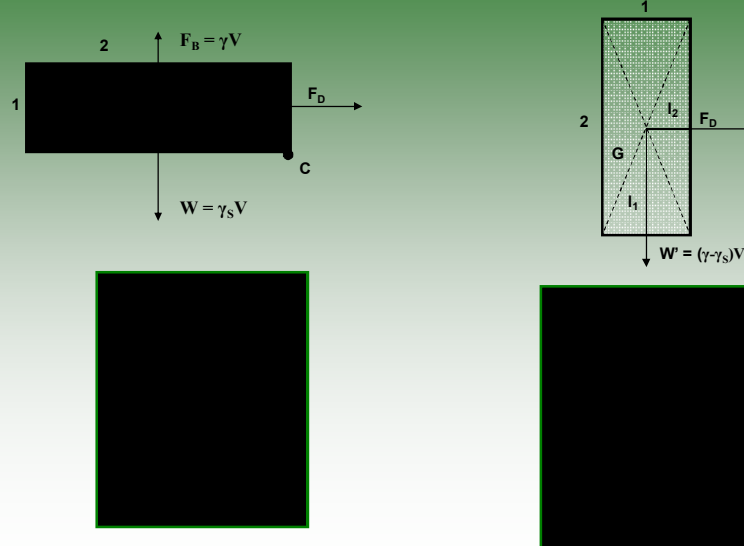


3. Incipient Motion

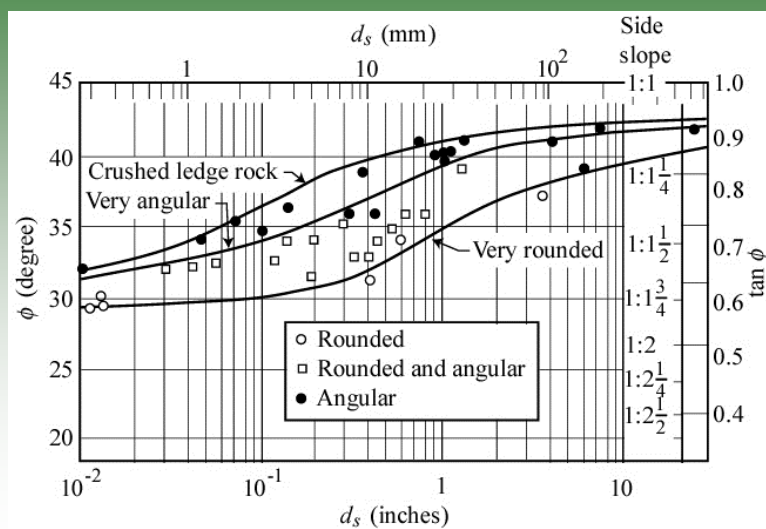
Angle of Repose Effects of Angularity



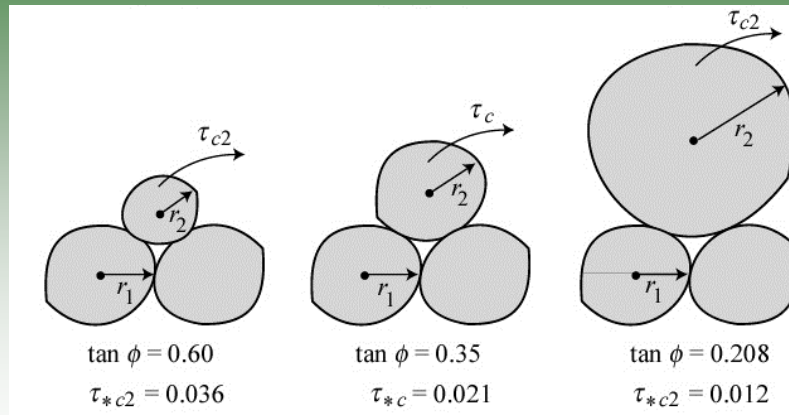
Angle of Repose Particle Orientation



Angle of Repose Granular Material



Angle of Repose Boundary and Particle Size



Los Corales



Incipient Motion



Shields Parameter

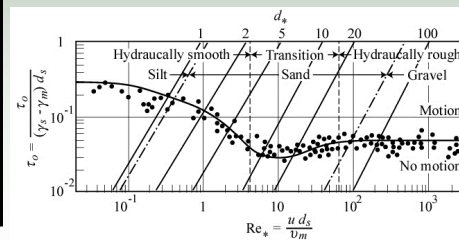
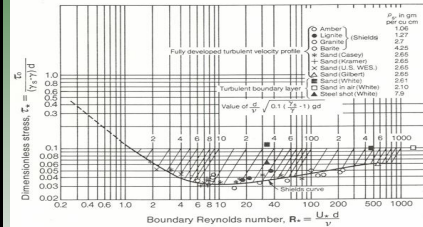
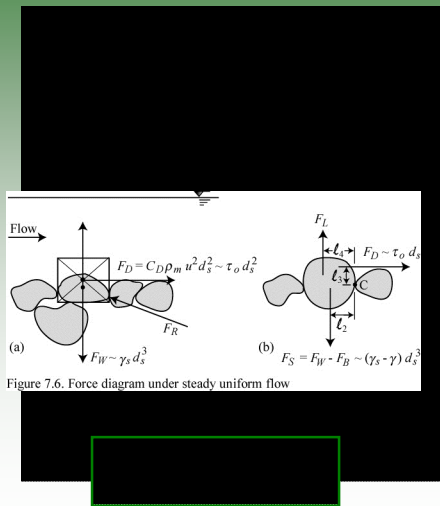
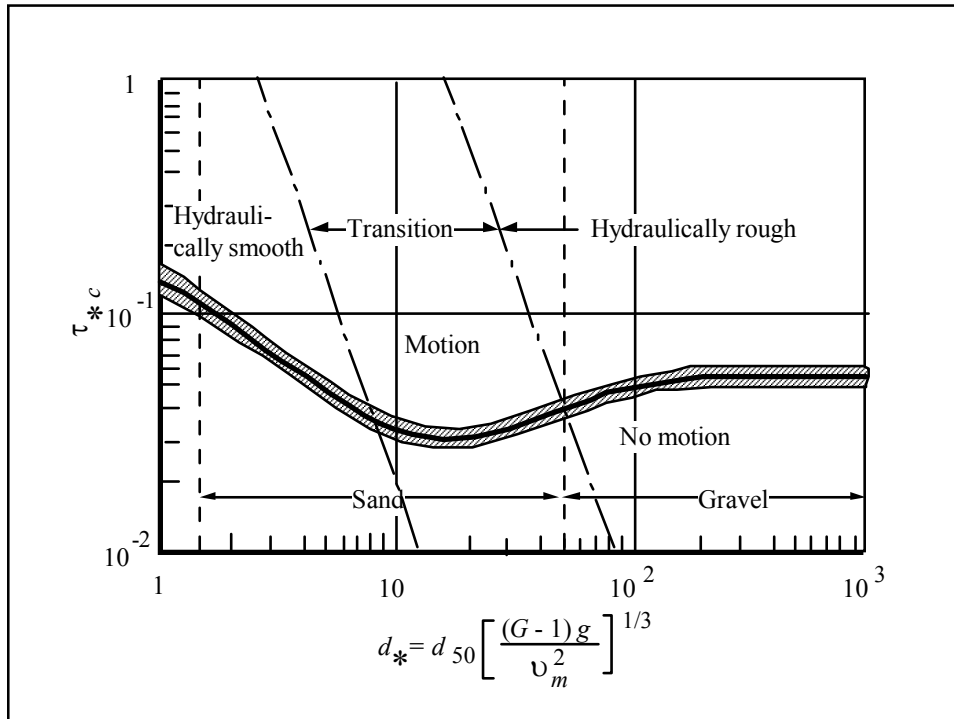


Figure 7.7. Shields diagram for granular material



Beginning of Motion

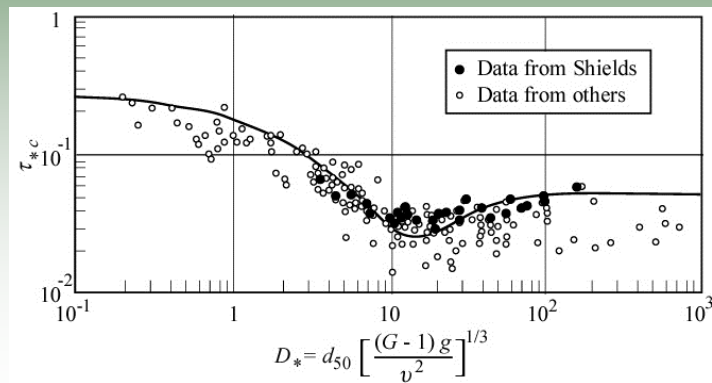


Figure 7.8. Modified Shields diagram

Calculating Critical Shear Stress

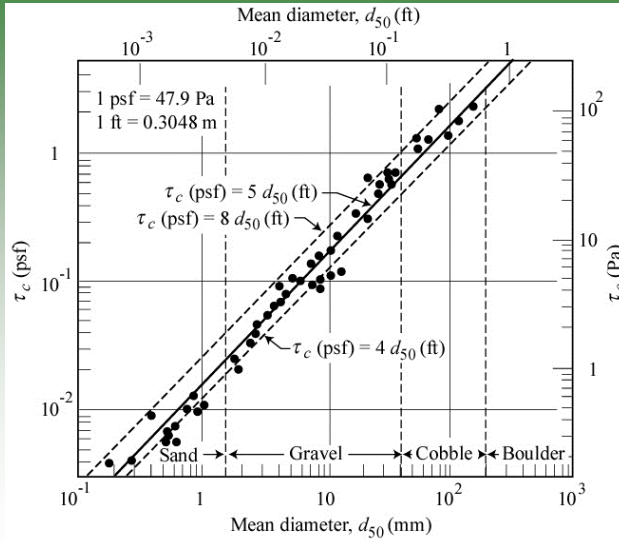
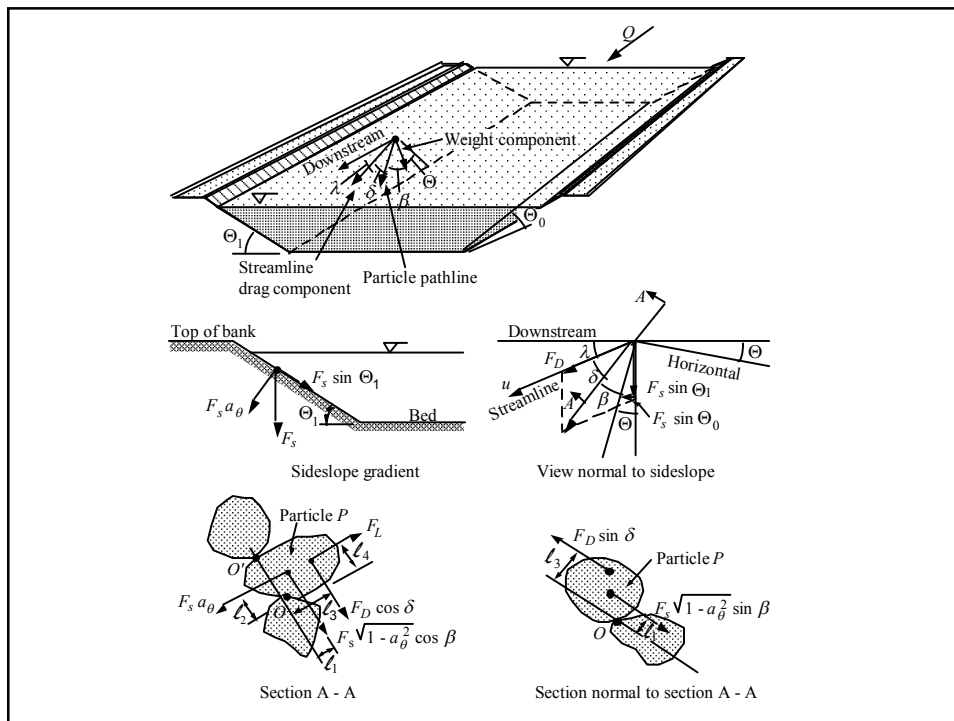


Figure 7.9. Critical shear stress on a horizontal surface



4. Bedforms

What are bedform?

Initiation of Bedforms on a flat bed

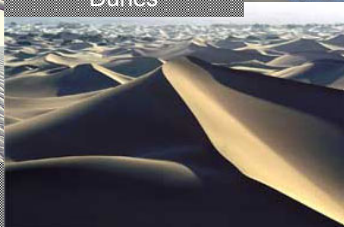


Ripples



Wind Blown
Ripples on Dunes

Dunes



Antidunes

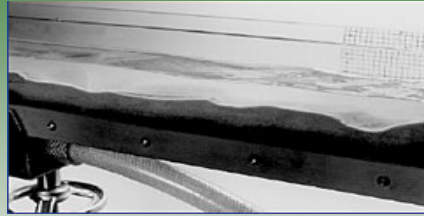
Lower Regime - Ripples



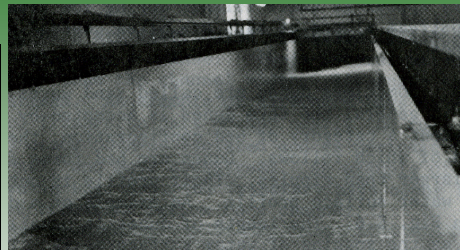
Lower Regime - Dunes



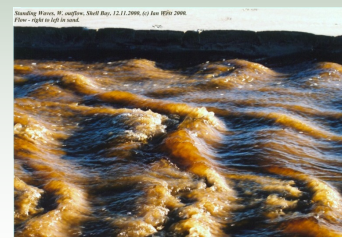
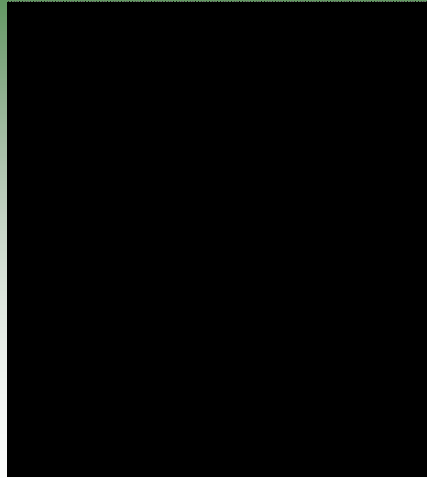
Transition - Washed Out Dunes



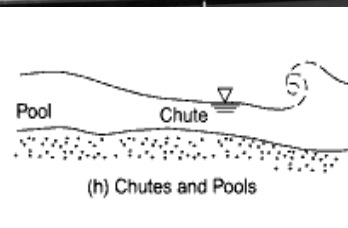
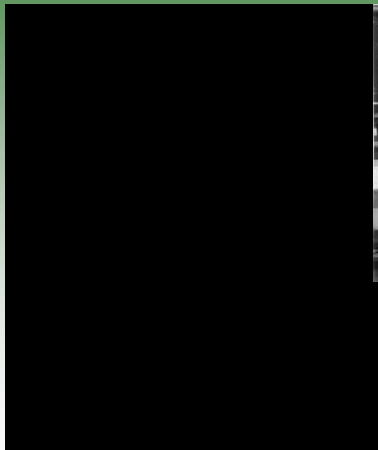
Upper Regime - Plane Bed

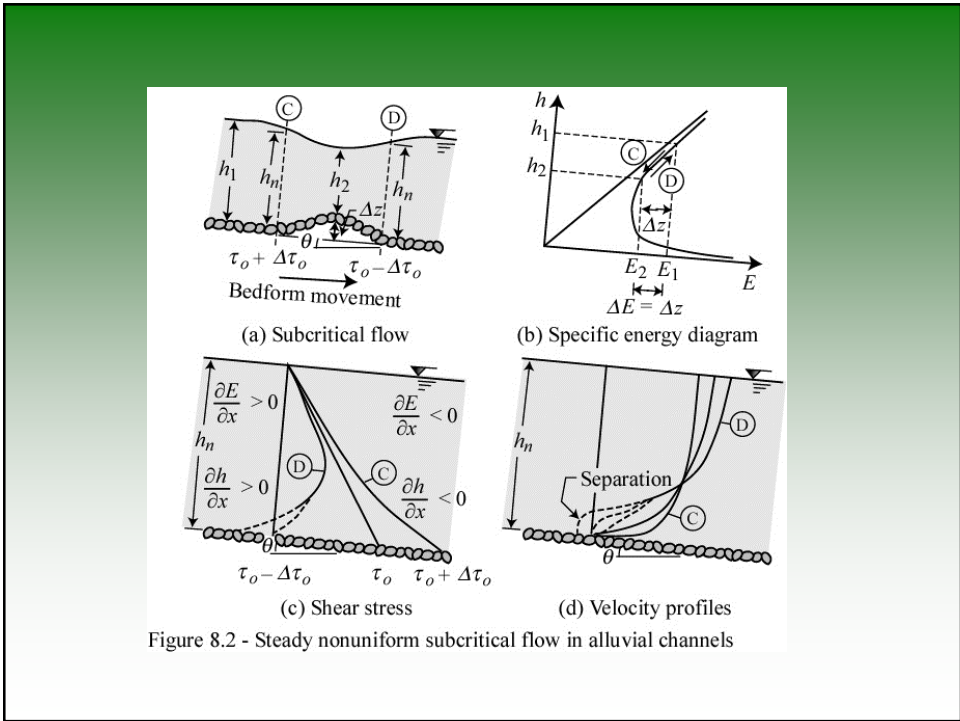


Upper Regime – Antidunes

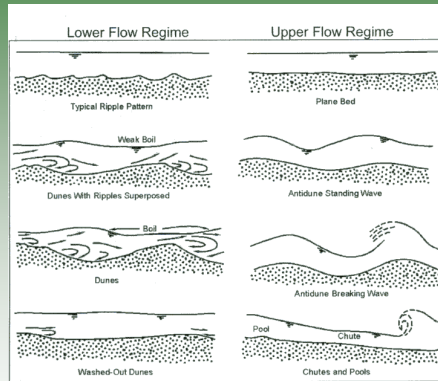
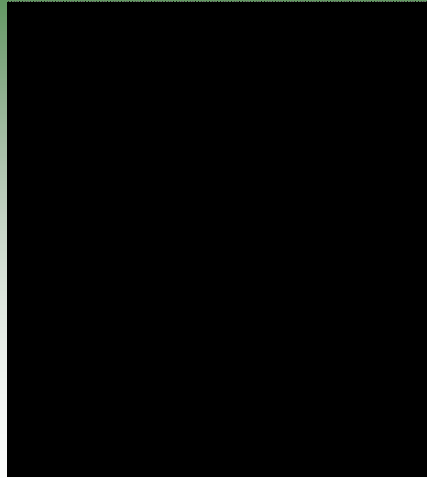


Upper Regime – Chutes and Pools



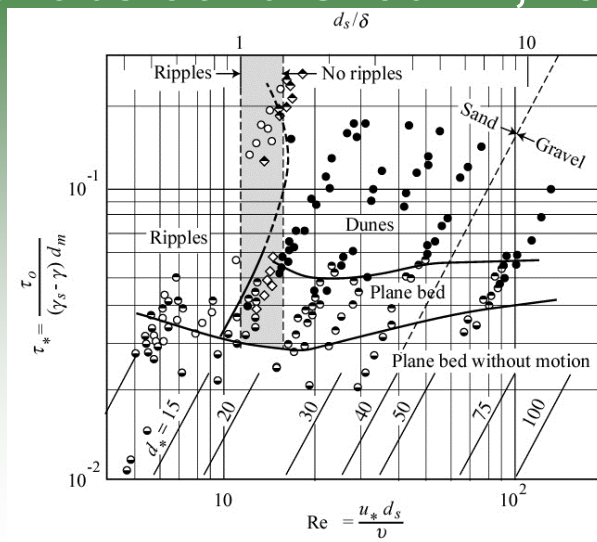


Bedform Classification

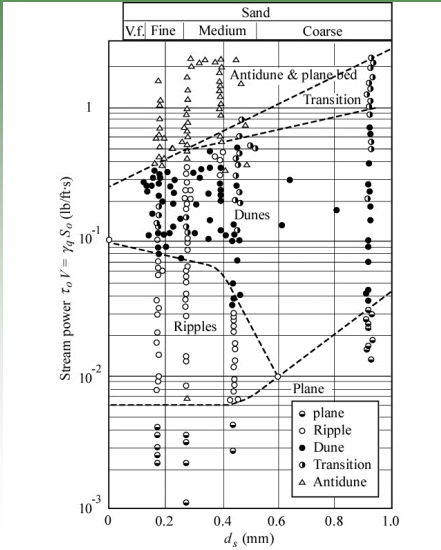


Forms of bed roughness in sand channels
(Simons and Richardson 1963, 1966)

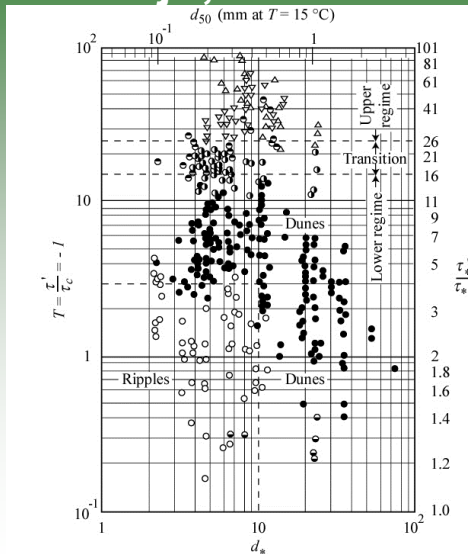
Based on the Shields Number Chabert and Chauvin, 1963



Based on Stream Power Simons and Richardson, 1963, 1966



Based on Transport-Stage Parameter van Rijn, 1984b



Laboratory Bedform Geometry from van Rijn, 1984

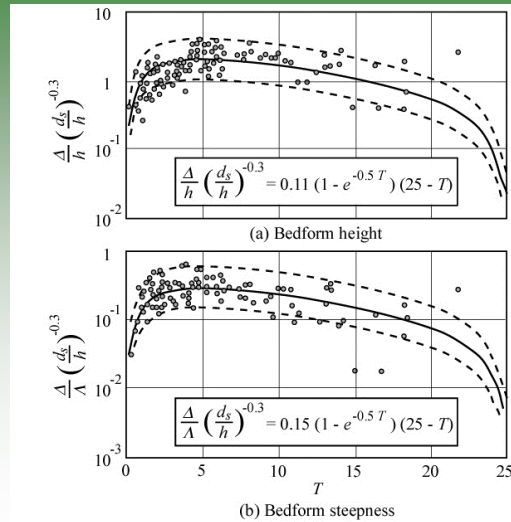


Figure 8.11. Bedform height and steepness (after van Rijn, 1984b)

Bedform Profiles

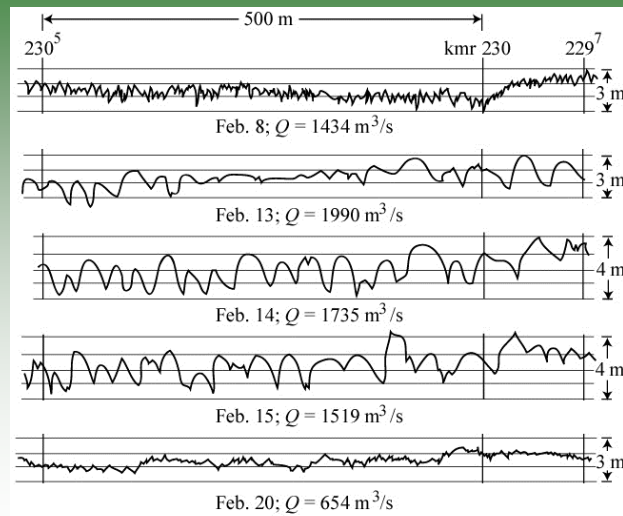
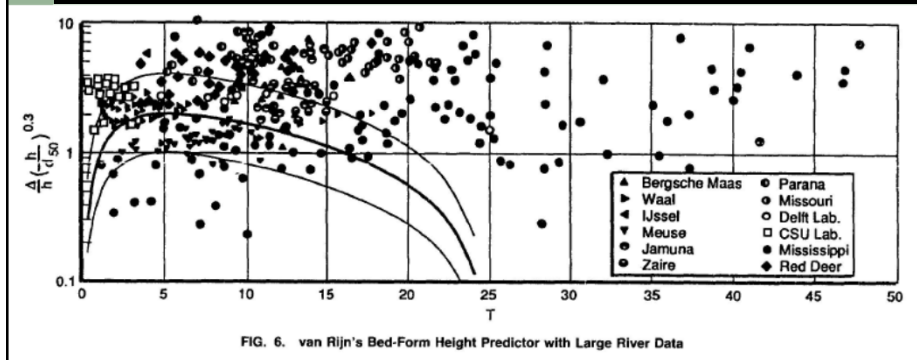
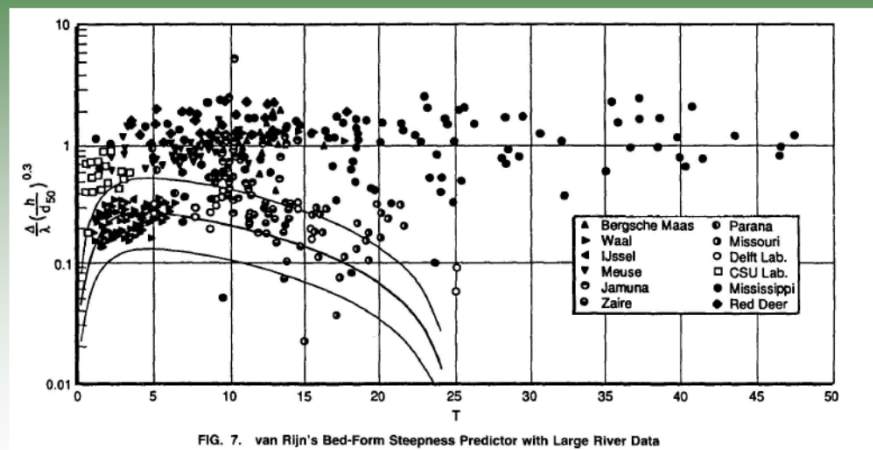


Figure 8.15. Dunes of the Bergsche Maas during the 1984 flood of the Meuse River (kmr denotes river kilometer; after Adriaanse, 1986)

Dune Height from Large Rivers

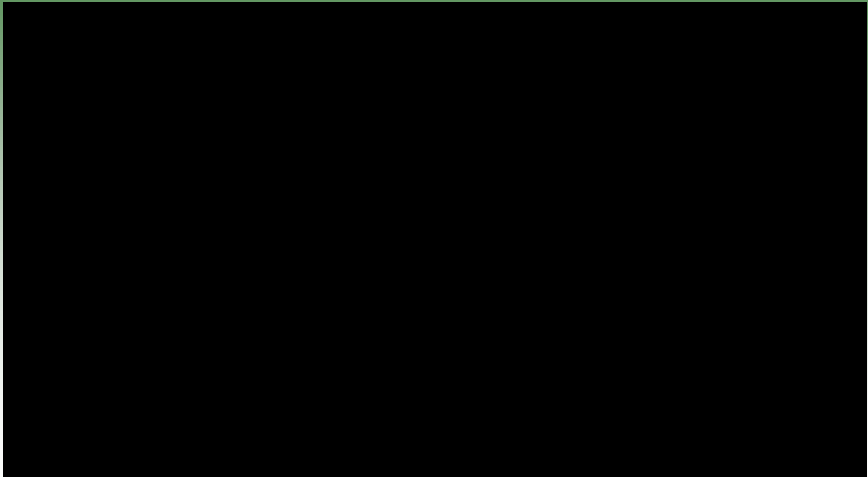


Dune Steepness from Large Rivers



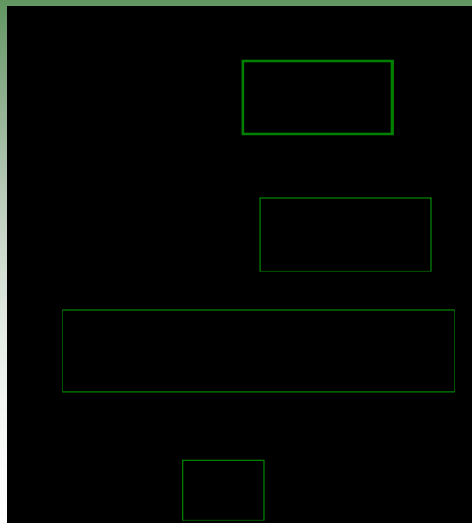
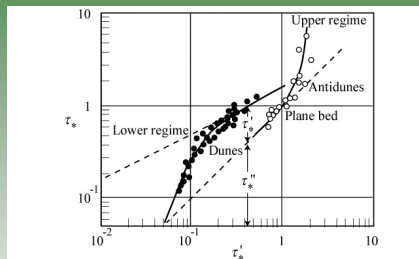


Resistance to Flow

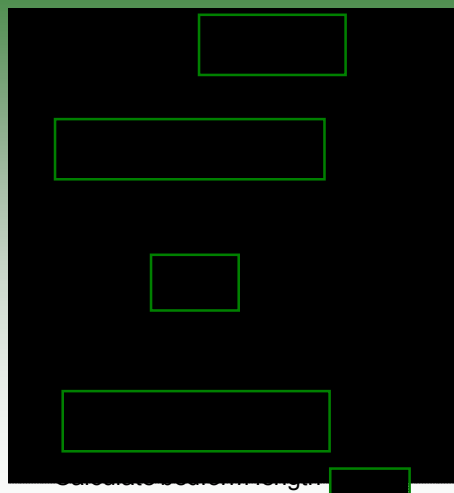
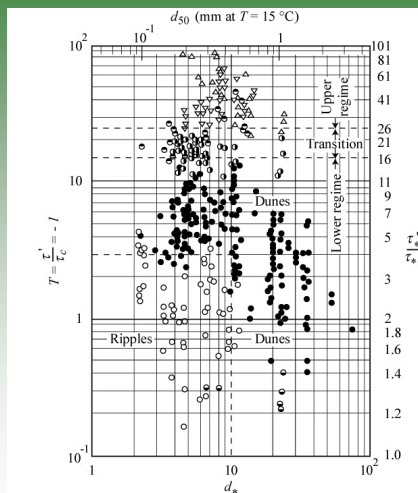


$$\tau_* = \tau_*' + \tau_*''$$

Engelund's Resistance Method



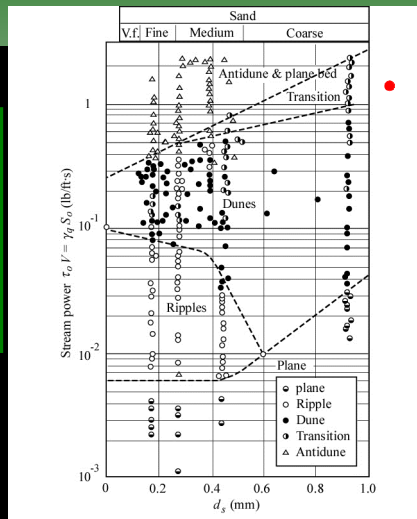
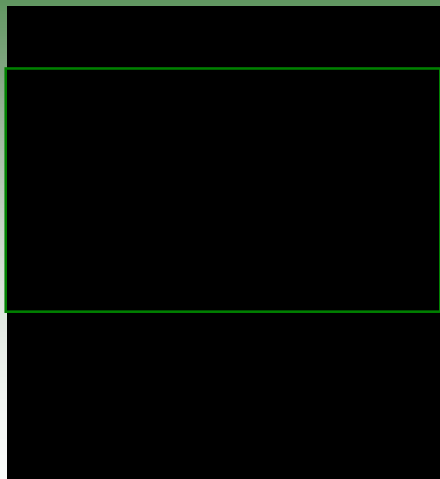
van Rijn's Resistance Method



Example - Rhine River

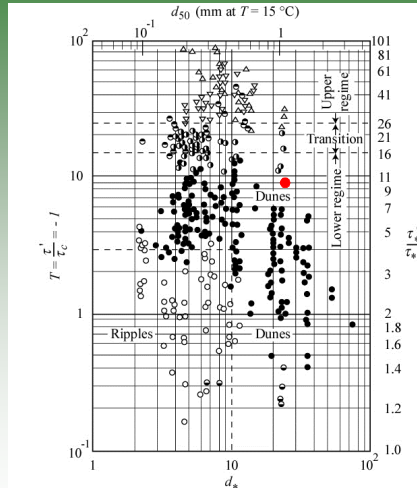
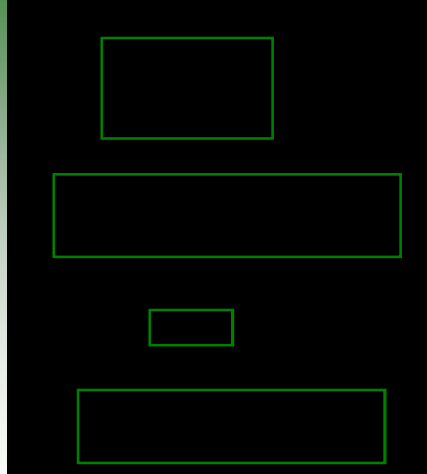


Example – Bedform Predictor



Simons and Richardson's method predicts washed-out dunes

Example - van Rijn



Based on result the expected bed forms are **DUNES**. In the field, dunes 0.9 m high and 20 m long were observed.

Example - Determine τ_*'

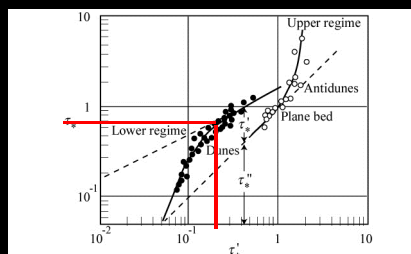
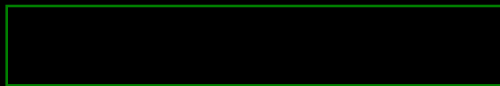
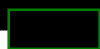
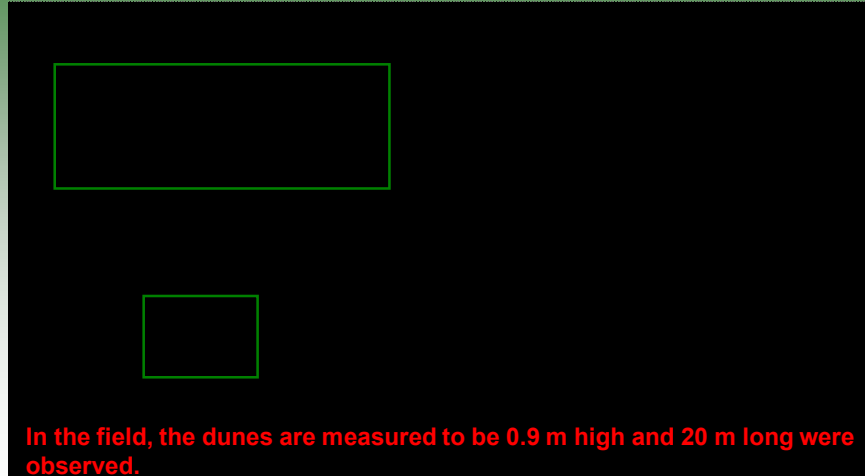


Figure 8.12. Total versus grain resistance (after Engelund and Hansen, 1967).



Example – Dune Height and Length



Rhine River Flood 1998

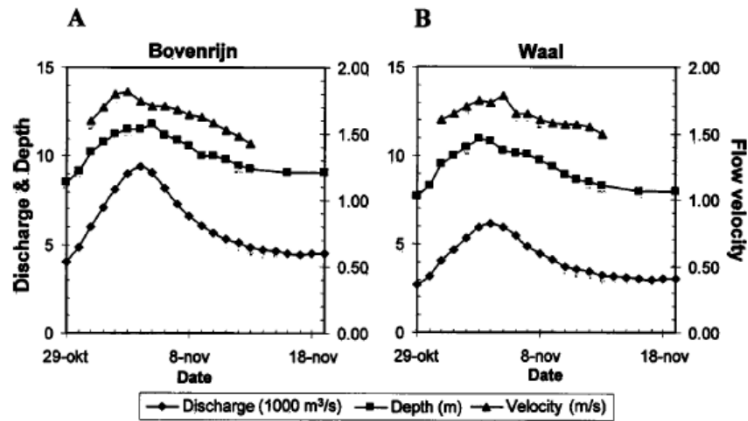
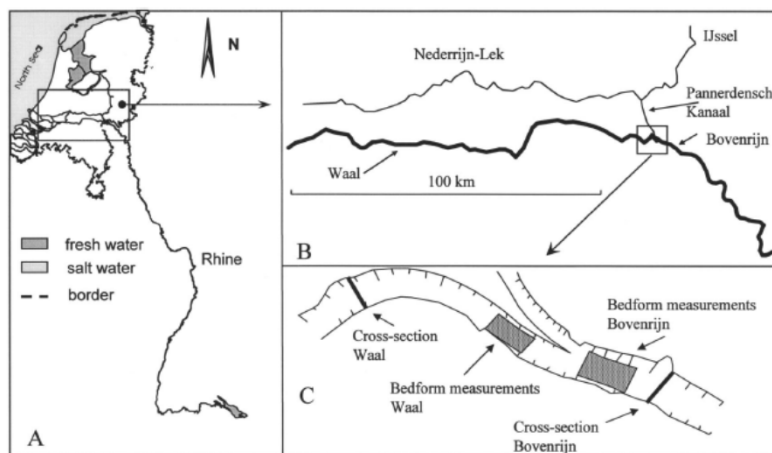


Fig. 2. Flood discharge, flow depth, and velocity: (a) Bovenrijn and (b) Waal

Study Area of the Rhine River



Field Surveys on the Rhine River



Bedform Profiles

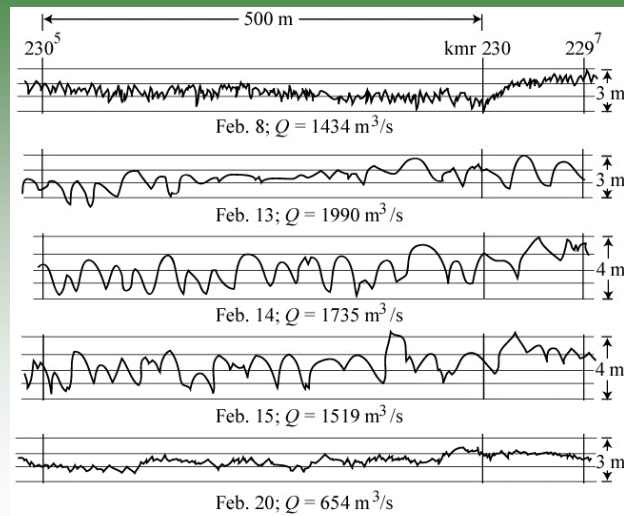


Figure 8.15. Dunes of the Bergsche Maas during the 1984 flood of the Meuse River (kmr denotes river kilometer; after Adriaanse, 1986)

Dune Height during the Flood

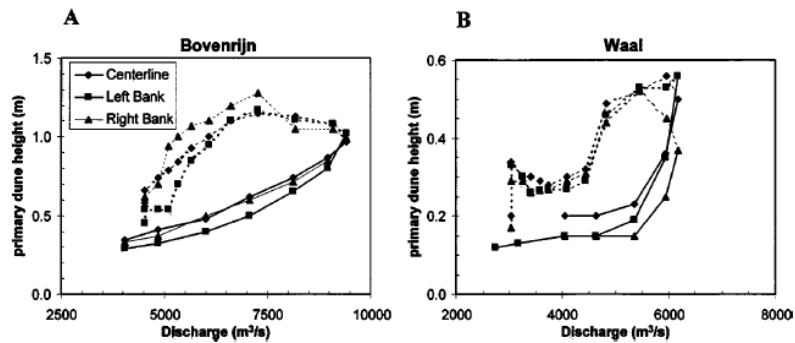


Fig. 3. Primary dune height versus discharge: (a) Bovenrijn and (b) Waal (dashed lines represent falling stage)

Manning n during the Flood

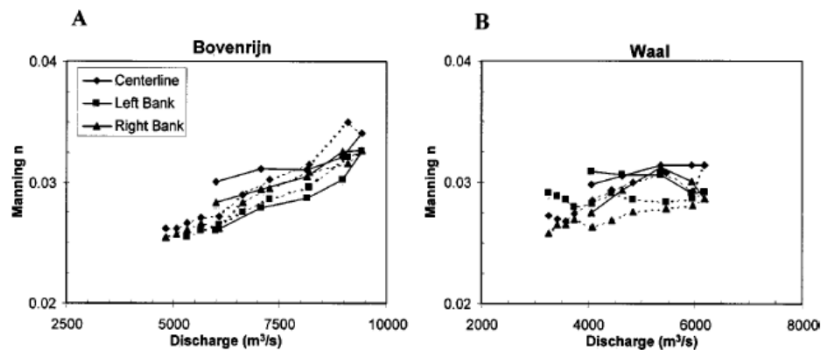
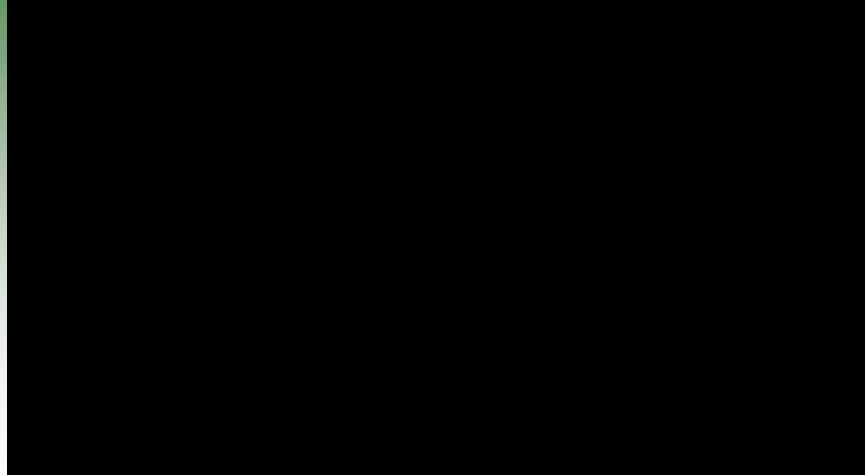
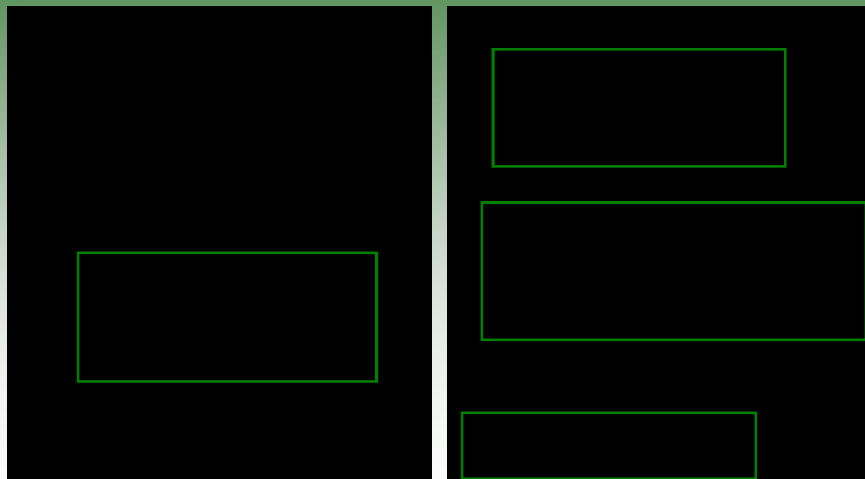


Fig. 6. Manning n versus discharge: (a) Bovenrijn and (b) Waal (dashed lines correspond to falling stage)

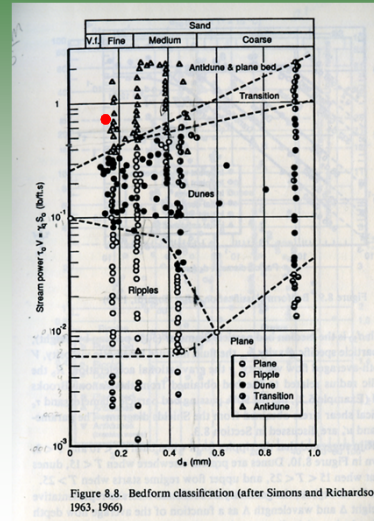
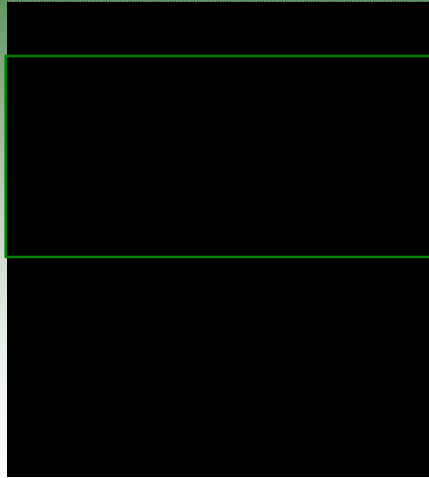
Example Amazon River



Calculate Depth and Velocity



Simons and Richardson



van Rijn

